



FINAL PROJECT - RC 141501

FEASIBILITY STUDY OF BULU ROAD WIDENING KM SBY 110+500 - 118+000 TUBAN DISTRICT EAST JAVA

ARIF ILMAWAN HS
NRP. 3111 106 023

SUPERVISORS
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CIVIL ENGINEERING DEPARTMENT
Faculty of Civil Engineering and Planning
Institut Teknologi Sepuluh Nopember
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TUGAS AKHIR - RC 141501

STUDI KELAYAKAN PELEBARAN JALAN BULU KM SBY 110+500 - 118+000 KABUPATEN TUBAN JAWA TIMUR

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TUGAS AKHIR

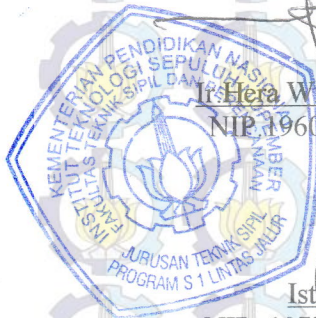
Diajukan Untuk Memenuhi Salah Satu Syarat
Memperoleh Gelar Sarjana Teknik
pada

Program Studi S-1 Jurusan Teknik Sipil
Fakultas Teknik Sipil dan Perencanaan
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JANUARI, 2015

**FEASIBILITY STUDY OF BULU ROAD WIDENING
KM SBY 110+500 - 118+000 TUBAN DISTRICT EAST JAVA**

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Tuban district is included in area that passed by the route of north coast of Java, which is the main route is national road that connect inter-provincial. So that the pantura road have a very high traffic. This matter makes Tuban district as a high attraction area and cause the traffic increasing. The increasing of traffic volume is caused by the increasing of the volume of vehicles that exceed the capacity of the road which exist and switching the function of the road. The dense flow of vehicles caused congestion in some segment of the road that occur at rush hour. This resulted in cost overruns which must be paid by the road users.

The purpose of this final project is to analyze whether it is feasible if the segment of the road perform widening, which is reviewed in term of traffic and economy highway. Analysis that will be calculated among others is analysis of the performance level in the value of the degree of saturation, analysis of the time value and vehicle operation cost, analytical calculation of the benefit cost ratio and net present value calculation analysis.

Which will be determined based on the value of the vehicle operating cost savings and time value. The vehicle

operating cost and the value of time and the value of the plan by using the method of Jasa Marga.the result of economic analysis obtained by value saving is Rp. 292.789.865.219,009 Value of BCR = 2,07 (BCR> 1) and value of NPV= Rp. 108.782.277.806,73 (NPV> 0) this project declare feasible.

Key Word : Bulu Road – Tuban, Feasibility study, BOK

**STUDI KELAYAKAN PELEBARAN JALAN
BULU KM SBY 110+500 – 118+000 KABUPATEN
TUBAN JAWA TIMUR**

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ABSTRAK

Kabupaten Tuban termasuk daerah yang dilalui jalur pantai utara jawa (pantura) yang merupakan jalur utama perdagangan di pulau jawa. Jalur pantura merupakan jalan nasional yang menghubungkan antar provinsi sehingga jalur pantura memiliki lalu lintas yang sangat padat. Hal ini menjadikan kabupaten Tuban sebagai daerah tarikan yang tinggi dan mengakibatkan lalu lintas yang terjadi semakin meningkat. Meningkatnya volume lalu - lintas dipengaruhi oleh meningkatnya jumlah kendaraan sehingga melebihi kapasitas jalan yang ada dan beralihnya fungsi jalan. Arus kendaraan yang padat mengakibatkan kemacetan di beberapa ruas jalan yang terjadi pada jam sibuk . Hal ini mengakibatkan pembengkakan biaya yang harus dikeluarkan oleh pengguna jalan.

Tujuan dari tugas Akhir ini untuk menganalisa apakah layak jika di ruas jalan tersebut dilakukan pelebaran yang ditinjau dari segi lalu lintas dan ekonomi jalan raya. Analisa yang akan dihitung antara lain analisa tingkat kinerja jalan berupa nilai Derajat Kejenuhan (DS), analisa perhitungan penghematan Nilai Waktu dan Biaya Operasional Kendaraan, analisa perhitungan Benefit Cost Ratio (BCR), dan analisa perhitungan Net Present Value (NPV).

Analisa kelayakan pelebaran jalan Bulu Tuban ditinjau dari segi ekonomi yang akan ditentukan berdasarkan nilai penghematan BOK dan nilai waktu. BOK dan nilai waktu dianalisa pada saat kondisi eksisting dan kondisi rencana dengan menggunakan metode Jasa Marga. Hasil analisa dari segi ekonomi diperoleh nilai saving Rp. 292.789.865.219,009 nilai BCR = 2,07 (BCR>1) dan nilai NPV = Rp. 108.782.277.806,73 (NPV>0) proyek dikatakan layak dilaksanakan.

Kata kunci : jalan Bulu – Tuban, studi kelayakan, BOK

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BAB I PENDAHULUAN

1.1 Latar Belakang

Kabupaten Tuban termasuk daerah yang dilalui jalur pantai utara Jawa (pantura) yang merupakan jalur utama perdagangan di Pulau Jawa. Jalur pantura merupakan jalan nasional yang menghubungkan antar provinsi sehingga jalur pantura memiliki lalu lintas yang sangat padat. Hal ini menjadikan Kabupaten Tuban sebagai daerah tarikan yang tinggi dan mengakibatkan lalu lintas yang terjadi semakin meningkat. Meningkatnya volume lalu - lintas dipengaruhi oleh meningkatnya jumlah kendaraan sehingga melebihi kapasitas jalan yang ada dan beralihnya fungsi jalan, sehingga jalan tidak berfungsi sebagaimana mestinya. Arus kendaraan yang padat mengakibatkan kemacetan di beberapa ruas jalan yang terjadi pada jam sibuk . Hal ini mengakibatkan pembengkakan biaya yang harus dikeluarkan oleh pengguna jalan.

Pemerintah provinsi melakukan berbagai macam alternatif untuk mengurangi kepadatan di Jalur Pantura Bulu Tuban. Untuk mengatasi masalah tersebut Direktorat Jendral Bina Marga sebagai salah satu instansi yang terkait sedang melaksanakan proyek pelebaran Jalan Bulu Tuban.

Pada tugas akhir ini penulis menganalisa kelayakan pelebaran jalan Bulu Tuban, apakah layak jika di ruas jalan tersebut dilakukan pelebaran yang ditinjau dari segi lalu lintas dan ekonomi jalan raya. Analisa yang akan dihitung antara lain analisa tingkat kinerja jalan berupa nilai derajat kejenuhan (DS), analisa perhitungan penghematan nilai waktu dan biaya operasional kendaraan, analisa perhitungan benefit cost ratio (BCR), dan analisa perhitungan net present value (NPV). Pelebaran jalan ini diharapkan memberikan kenyamanan bagi para pengguna jalan dilihat dari segi waktu dan biaya

Pelebaran Jalan Bulu Tuban yang mencakup Kecamatan Jenu, dan Kecamatan Tambakboyo ini bertujuan untuk mengurangi

kemacetan bagi pengguna jalan dari arah Kabupaten Tuban yang akan menuju Jawa Tengah atau sebaliknya. Pelebaran jalan ini diharapkan dapat meminimalkan kemacetan pergerakan lalu lintas di sekitarnya.

1.2 Rumusan Masalah

Dalam tugas akhir ini, rumusan masalah yang akan ditinjau adalah sebagai berikut :

1. Bagaimana kondisi dan karakteristik lalu lintas ruas jalan masuk dan keluar kabupaten Tuban sebelum ada rencana pelebaran Jalan Bulu Tuban?
2. Berapa penghematan biaya operasional kendaraan (BOK) dan nilai waktu setelah adanya pelebaran Jalan Bulu Tuban?
3. Bagaimana tingkat kelayakan pelebaran Jalan Bulu Tuban?

1.3 Tujuan

Tujuan dari tugas akhir ini adalah sebagai berikut :

1. Menganalisa bagaimana kondisi dan karakteristik lalu lintas ruas jalan masuk dan keluar Kabupaten Tuban sebelum ada rencana pelebaran Jalan Bulu Tuban.
2. Menganalisa penghematan biaya operasional kendaraan (BOK) dan nilai waktu setelah ada pelebaran Jalan Bulu Tuban.
3. Menganalisa tingkat kelayakan pelebaran Jalan Bulu Tuban.

1.4 Batasan Masalah

Untuk menghindari penyimpangan pembahasan dari masalah yang dibahas tugas akhir, maka diperlukan pembatasan masalah di antaranya :

1. Peninjauan lalu lintas hanya pada kapasitas, tingkat kinerja, dan analisa volume yang meliputi: derajat kejenuhan dan kecepatan.
2. Tidak menghitung kerugian atau peningkatan hasil produksi di daerah studi, mengenai pembebasan lahan, dan bidang social.

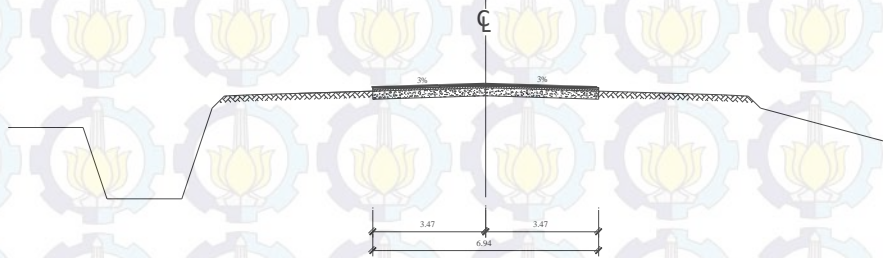
1.5 Lokasi Studi



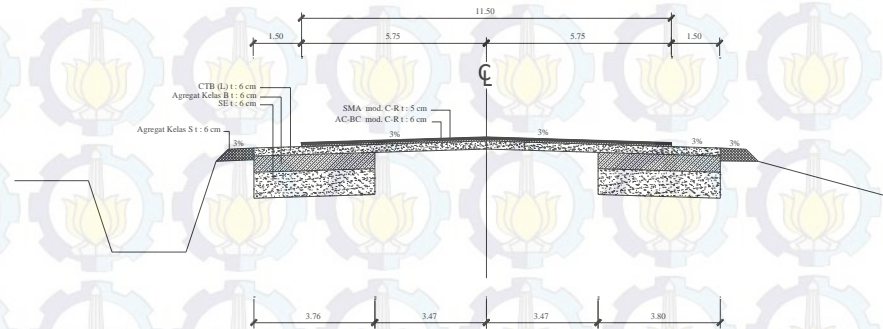
Gambar 1.1 Peta Jawa Timur
(sumber www.google.com)



Gambar 1.2 Peta Lokasi Studi
(sumber www.google.com 7 Maret 2014)



Gambar 1.3 Gambar Potongan Melintang KM 111 + 850 Eksisting
(sumber : Dinas Bina Marga Provinsi Jawa Timur)



Gambar 1.4 Gambar Potongan Melintang KM 111 + 850 Rencana
(sumber : Dinas Bina Marga Provinsi Jawa Timur)

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BAB II

TINJAUAN PUSTAKA

2.1 Umum

Dalam penulisan tugas akhir memerlukan suatu dasar teori sebagai pembahasan keseluruhan masalah. Dasar teori ini nantinya berisikan teori penunjang penulisan yang ditemukan oleh para ahli dibidangnya masing-masing .

2.2. Karakteristik Jalan

2.2.1 Klasifikasi Jalan Raya

Dalam tata cara perencanaan geometrik jalan antar kota yang dikeluarkan oleh Dinas Bina Marga Direktorat Jenderal Bina Marga Tahun 1997. Jalan dikelompokkan menjadi 3 macam berdasarkan fungsinya yaitu :

1. Jalan arteri : Jalan yang melayani angkutan utama dengan ciri-ciri perjalanan jarak jauh, kecepatan rata – rata tinggi, dan jumlah jalan masuk dibatasi.
2. Jalan kolektor : Jalan yang melayani angkutan pengumpulan dengan ciri – ciri perjalanan jarak sedang kecepatan rata – rata, dan jumlah jalan masuk dibatasi.
3. Jalan lokal : Jalan yang melayani angkutan setempat dengan ciri – ciri perjalanan jarak dekat, kecepatan rata – rata rendah, dan jumlah jalan masuk tidak dibatasi.

2.2.2 Tipe Jalan

Tipe jalan digunakan untuk menganalisa kapasitas jalan, menurut MKJI 1997 tipe jalan dibagi menjadi 4 (empat) yaitu :

1. Jalan dua lajur dua arah tak terbagi (2/2 UD) : Tipe jalan ini meliputi semua jalan perkotaan dua lajur dua arah (2/2 UD) dengan lebar lajur lalu lintas lebih kecil dan sama dengan 10,5 m atau sampai dengan 11 m. Kondisi dasar tipe jalan ini didefinisikan sebagai berikut :
 - Tidak ada median.
 - Pemisahan arah lalu lintas 50 – 50.

- Tipe alinyemen : datar.
- Guna lahan : tidak ada pengembangan samping jalan.
- Kelas hambatan samping : rendah / L .
- Kelas jarak pandang : A.
- Lebar jalur lalu lintas efektif 7 m.
- Lebar efektif bahu 1,5 m pada masing – masing sisi .

2. Jalan empat lajur dua arah tak terbagi (4/2 UD) : Tipe jalan ini meliputi semua jalan dua arah tak terbagi dengan marka lajur untuk empat lajur dan lebar jalur lalu lintas tak terbagi antara 12 m dan 15 m. Kondisi dasar tipe jalan ini didefinisikan sebagai berikut :

- Tidak ada median.
- Pemisahan arah lalu lintas 50 – 50.
- Tipe alinyemen : datar.
- Guna lahan : tidak ada pengembangan samping jalan.
- Kelas hambatan samping : rendah / L .
- Kelas jarak pandang : A.
- Lebar jalur lalu lintas efektif 7 m.
- Lebar efektif bahu 1,5 m pada masing – masing sisi .

3. Jalan empat lajur dua arah terbagi (4/2 D) : Tipe jalan ini meliputi semua jalan dua arah dengan dua lajur lalu lintas yang dipisahkan oleh median. Setiap jalur lalu lintas mempunyai dua lajur, bermarka dengan lebar antara 3,0 m – 3,75 m. Kondisi dasar tipe jalan ini didefinisikan sebagai berikut :

- Median.
- Pemisahan arah lalu lintas 50 – 50.
- Tipe alinyemen : datar.
- Guna lahan : tidak ada pengembangan samping jalan.
- Kelas hambatan samping : rendah / L .
- Kelas jarak pandang : A.

- Lebar jalur lalu lintas 2 x 7,0 m (tidak termasuk lebar median).
- Lebar efektif bahu 2,0 m diukur sebagai lebar bahu dalam ditambah bahu luar untuk setiap jalur lalu lintas .

4. Jalan enam lajur dua arah terbagi (6/2 D) : Tipe jalan ini meliputi semua jalan dua arah dengan lebar jalur lalu lintas lebih dari 18 m dan kurang dari 24 m. Kondisi dasar tipe jalan ini didefinisikan sebagai berikut :

- Median.
- Pemisahan arah lalu lintas 50 – 50.
- Tipe alinyemen : datar.
- Guna lahan : tidak ada pengembangan samping jalan.
- Kelas hambatan samping : rendah / L .
- Kelas jarak pandang : A.
- Lebar jalur lalu lintas 2 x 7,0 m (tidak termasuk lebar median).
- Lebar efektif bahu 2,0 m diukur sebagai lebar bahu dalam ditambah bahu luar untuk setiap jalur lalu lintas .

2.3 Karakteristik Lalu Lintas

2.3.1 Kapasitas

Sebagai arus maksimal yang dapat dipertahankan per satuan jam yang melewati suatu titik jalan dalam kondisi tertentu. Persamaan untuk menentukan kapasitas suatu jalan dengan alinyemen umum menurut MKJI 1997 adalah :

$$C = C_o \times F_{Cw} \times F_{Csp} \times F_{Csf} \text{ (smp / jam)} \dots\dots\dots \text{Pers. 2.1}$$

Dimana :

C = kapasitas

C_o = kapasitas dasar

F_{Cw} = faktor penyesuaian akibat lebar jalur lalu lintas

F_{Csp} = faktor penyesuaian akibat pemisahan arah

F_{Csf} = faktor penyesuaian akibat hambatan samping

Tabel 2.1 Kapasitas Dasar Pada Jalan Luar Kota 4-Lajur 2-Arah Tak Terbagi (4/2)

Tipe Jalan / Alinyemen Jalan	Kapasitas Dasar Untuk Satu Lajur (smp / jam / lajur)
Empat Lajur Terbagi	
Datar	1900
Bukit	1850
Gunung	1800
Empat Lajur Tak terbagi	
Datar	1700
Bukit	1650
Gunung	1600

Sumber : MKJI (1997)

Tabel 2.2 Kapasitas dasar pada jalan luar kota (2/2 UD) (C_0)

Tipe jalan/ Tipe alinyemen	Kapasitas dasar Total kedua arah smp/jam
Dua-lajur tak-terbagi	
- Datar	3100
- Bukit	3000
- Gunung	2900

Sumber : MKJI (1997)

Tabel 2.3 Faktor Penyesuaian Kapasitas Akibat Lebar Jalur Lalu Lintas (F_{Cw})

Tipe Jalan	Lebar Efektif Jalur Lalu-lintas (Wc/m)	FCw
Empat – lajur terbagi	Per lajur	
	3	0.91
Enam – lajur terbagi	3.25	0.96
	3.5	1
	3.75	1.03
Empat – lajur tak terbagi	3	0.91
	3.25	0.96
	3.5	1
Dua – lajur tak terbagi	3.75	1.03
	6	0.91
	7	1
	8	1.08
	9	1.15
	10	1.21
	11	1.27

Sumber : MKJI (1997)

Tabel 2.4 Faktor Penyesuaian Kapasitas Akibat Pemisah Arah (F_{Csp})

Pemisahan arah SP %-%		50-50	55-45	60-40	65-35	70-30
F _{Csp}	Dua-lajur 2/2	1.00	0.97	0.94	0.91	0.88
	Empat-lajur 4/2	1.00	0.975	0.95	0.925	0.90

Sumber : MKJI (1997)

Tabel 2.5 Faktor Penyesuaian Kapasitas Akibat Hambatan Samping (F Csf)

Tipe jalan	Kelas hambatan samping	Faktor penyesuaian akibat hambatan samping (FC_{sf})			
		Lebar bahu efektif W_e			
		$\leq 0,5$	1,0	1,5	$\geq 2,0$
4/2 D	VL	0,99	1,00	1,01	1,03
	L	0,96	0,97	0,99	1,01
	M	0,93	0,95	0,96	0,99
	H	0,90	0,92	0,95	0,97
	VH	0,88	0,90	0,93	0,96
2/2 UD	VL	0,97	0,99	1,00	1,02
	L	0,93	0,95	0,97	1,00
4/2 UD	M	0,88	0,91	0,94	0,98
	H	0,84	0,87	0,91	0,95
	VH	0,80	0,83	0,88	0,93

Sumber : MKJI (1997)

2.3.2 Volume lalu lintas

Jumlah kendaraan yang melewati satu titik pengamatan pada suatu jalur jalan selama satu satuan waktu. Untuk mendapatkan volume lalu lintas dilakukan survey volume lalu lintas.

- Sedan, jeep, dan station wagon
- Oplet, pick up suburban dan combi (penumpang)
- Micro truck dan mobil penumpang
- Bis kecil
- Bis besar
- Truk 2 as
- Truk Tangki 2 As > 10 T
- Truk Tangki gandengan
- Truk 3 As atau lebih

Dari hasil survey volume lalu lintas dapat diketahui :

- Komposisi arus lalu lintas
- Lalu lintas Harian Rata-rata (LHR)

2.3.3 Derajat Kejenuhan

Rasio arus terhadap kapasitas yang digunakan sebagai faktor kunci dalam penentuan perilaku lalu lintas pada suatu simpang dan juga segmen jalan. Menurut MKJI 1997 nilai derajat kejenuhan menunjukkan apakah segmen jalan akan mempunyai masalah / tidak. Derajat kejenuhan ini diberi batasan maksimum = 0,75 ; bila melebihi dari 0,75 maka dianggap jalan sudah tidak mampu lagi menampung arus lalu lintas.

$$D_s = \frac{Q}{C} \dots\dots\dots \text{Pers. 2.2}$$

Dimana :

D_s = derajat kejenuhan

Q = arus total lalu lintas (smp/jam)

C = kapasitas

2.3.4 Kecepatan

Dihitung sebagai panjang jalan dibagi waktu tempuh jalan tersebut. Kecepatan tempuh digunakan sebagai ukuran utama kinerja segmen jalan serta masukan yang penting bagi pemakai jalan dalam analisa ekonomi.

$$V = \frac{L}{TT} \dots\dots\dots \text{Pers. 2.3}$$

Dimana :

V = kecepatan (km/jam)

TT = waktu tempuh rata – rata (jam)

L = jarak yang ditempuh (km)

2.3.5 Derajat Iringan

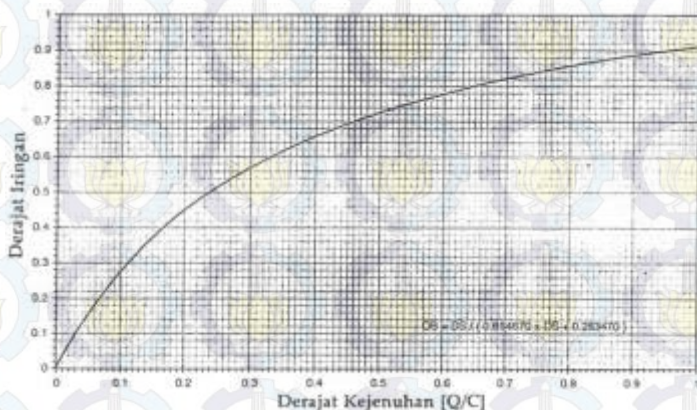
Derajat iringan yaitu rasio antara kendaraan didalam pleton terhadap arus total. Pleton didefinisikan sebagai gerakan dari kendaraan yang beriringan dengan waktu antara gandar depan ke gandar depan dari kendaraan yang didepannya dari setiap kendaraan.

$$DB = DS / (0,814670 \times DS + 0,283470) \dots \dots \dots \text{pers 2.4}$$

Dimana :

DB = Derajat iringan

DS = Derajat kejenuhan



Gambar 2.1 Grafik hubungan antara derajat kejenuhan dengan derajat iringan.

2.3.6 Ekuivalensi Mobil Penumpang

Faktor konversi berbagai jenis kendaraan dibandingkan dengan mobil penumpang atau kendaraan ringan lainnya sehubungan dengan dampaknya pada perilaku lalu lintas, dimana arus berbagai kendaraan yang berbeda telah diubah menjadi arus kendaraan ringan (termasuk mobil penumpang/emp) seperti yang tertulis dalam MKJI 1997.

Tabel 2.6 Tabel emp (2/2 UD)

Tipe Alinyemen	Arus Total (kend/jam)	emp					
		MHV	LB	LT	MC		
					Lebar jalur lalu lintas (m)		
					< 6 m	6 - 8 m	> 8 m
Datar	0	1,2	1,2	1,8	0,8	0,6	0,4
	800	1,8	1,8	2,7	1,2	0,9	0,6
	1350	1,5	1,6	2,5	0,9	0,7	0,5
	≥ 1900	1,3	1,5	2,5	0,6	0,5	0,4
Bukit	0	1,8	1,6	5,2	0,7	0,5	0,3
	650	2,4	2,5	5,0	1,0	0,8	0,5
	1100	2,0	2,0	4,0	0,8	0,6	0,4
	≥ 1600	1,7	1,7	3,2	0,5	0,4	0,3
Gunung	0	3,5	2,5	6,0	0,6	0,4	0,2
	450	3,0	3,2	5,5	0,9	0,7	0,4
	900	2,5	2,5	5,0	0,7	0,5	0,3
	≥ 1350	1,9	2,2	4,0	0,5	0,4	0,3

Sumber : MKJI (1997)

Tabel 2.7 Tabel emp (4/2 D)

Tipe Alinyemen	Arus Total (Kend/Jam)		Emp			
	Jalan Terbagi Per Arah (Kend/Jam)	Jalan Tak Terbagi Total (Kend/Jam)	MHV	LB	LT	MC
Datar	0	0	1,2	1,2	1,6	0,5
	1000	1700	1,4	1,4	2	0,6
	1800	3250	1,6	1,7	2,5	0,8
	≥ 2150	≥ 3950	1,3	1,5	2	0,5
Bukit	0	0	1,8	1,6	4,8	0,4
	750	1350	2	2	4,6	0,5
	1400	2500	2,2	2,3	4,3	0,7
	≥ 1750	≥ 3150	1,8	1,9	3,5	0,4
Gunung	0	0	3,2	2,2	5,5	0,3
	550	1000	2,9	2,6	5,1	0,4
	1100	2000	2,6	2,9	4,8	0,6
	≥ 1500	≥ 2700	2	2,4	3,8	0,3

Sumber : MKJI (1997)

2.3.7 Kecepatan Arus Bebas

Jarak yang ditempuh dalam waktu tertentu. Faktor yang mempengaruhi kecepatan adalah kondisi jalan, volume lalu lintas, kondisi kendaraan dan lingkungan. Sedangkan kecepatan kendaraan bebas adalah kecepatan pada saat tingkatan arus nol sesuai dengan kecepatan yang dipilih pengemudi. Seandainya mengendarai kendaraan bermotor, tanpa halangan kendaraan bermotor lainnya.

$$FV = (Fv_o + FV_w) \times FFV_{sf} \times FFV_{rc} \dots \dots \dots \text{Pers. 2.5}$$

Dimana :

FV = Kecepatan arus bebas kendaraan ringan pada kondisi lapangan (km/jam).

Fvo = Kecepatan arus bebas dasar kendaraan ringan (km/jam).

FFW = Penyesuaian untuk lebar efektif jalur lalu lintas (km/jam), penambahan.

FFVsf = Faktor penyesuaian untuk kondisi hambatan samping, perkalian.

FFVrc = Faktor penyesuaian untuk kelas fungsi jalan, perkalian.

Tabel 2.8 Kecepatan Arus Bebas Dasar (FVo)

Tipe jalan/ Tipe alinyemen/ (Kelas jarak pandang)	Kecepatan arus bebas dasar (km/jam)				
	Kendaraan ringan LV	Kendaraan berat menengah MHV	Bus besar LB	Truk besar LT	Sepeda motor MC
Enam-lajur terbagi					
- Datar	83	67	86	64	64
- Bukit	71	56	68	52	58
- Gunung	62	45	55	40	55
Empat-lajur terbagi					
- Datar	78	65	81	62	64
- Bukit	68	55	66	51	58
- Gunung	60	44	53	39	55
Empat-lajur tak terbagi					
- Datar	74	63	78	60	60
- Bukit	66	54	65	50	58
- Gunung	58	43	52	39	53
Dua-lajur tak terbagi					
- Datar SD: A	68	60	73	58	55
" " B	65	57	69	55	54
" " C	61	54	63	52	53
- Bukit	61	52	62	49	53
- Gunung	55	42	50	38	51

Sumber : MKJI (1997)

Tabel 2.9 Faktor Penyesuaian Kecepatan Arus Bebas Akibat Lebar Jalur Lalu Lintas (FVw)

Tipe jalan	Lebar efektif jalur lalu lintas (W_e) (w)	FVw (km/jam)		
		Datar: SDC= A,B	- Bukit: SDC= A,B,C - Datar: SDC=C	Gunung
Empat-lajur dan Enam-lajur terbagi	Per lajur			
	3,00	-3	-3	-2
	3,25	-1	-1	-1
	3,75	0	0	0
Empat-lajur tak terbagi	Per lajur			
	3,00	-3	-2	-1
	3,25	-1	-1	-1
	3,50	0	0	0
Dua-lajur tak terbagi	Total			
	5	-11	-9	-7
	6	-3	-2	-1
	7	0	0	0
	8	1	1	0
	9	2	2	1
10	3	3	2	
11	3	3	2	

Sumber : MKJI (1997)

Tabel 2.10 Faktor Penyesuaian Kecepatan Arus Bebas Akibat Hambatan Samping (FFVsf)

Tipe jalan	Kelas hambatan samping (SFC)	Faktor penyesuaian akibat hambatan samping dan lebar bahu			
		Lebar bahu efektif W_s (m)			
		$\leq 0,5$ m	1,0 m	1,5 m	≥ 2 m
Empat-lajur terbagi 4/2 D	Sangat rendah	1,00	1,00	1,00	1,00
	Rendah	0,98	0,98	0,98	0,99
	Sedang	0,95	0,95	0,96	0,98
	Tinggi	0,91	0,92	0,93	0,97
	Sangat Tinggi	0,86	0,87	0,89	0,96
Empat-lajur tak terbagi 4/2 UD	Sangat rendah	1,00	1,00	1,00	1,00
	Rendah	0,96	0,97	0,97	0,98
	Sedang	0,92	0,94	0,95	0,97
	Tinggi	0,88	0,89	0,90	0,96
	Sangat Tinggi	0,81	0,83	0,85	0,95
Dua-lajur tak terbagi 2/2 UD	Sangat rendah	1,00	1,00	1,00	1,00
	Rendah	0,96	0,97	0,97	0,98
	Sedang	0,91	0,92	0,93	0,97
	Tinggi	0,85	0,87	0,88	0,95
	Sangat Tinggi	0,76	0,79	0,82	0,93

Sumber : MKJI (1997)

Tabel 2.11 Faktor Penyesuaian Kecepatan Arus Bebas Akibat Kelas Fungsional Jalan (FFV_{rc})

Tipe Jalan	Faktor penyesuaian FFV _{rc}					
	Pengembangan samping jalan (%)					
	0	25	50	75	100	
Empat-lajur terbagi	Arteri	1,00	0,99	0,98	0,96	0,95
	Kolektor	0,99	0,98	0,97	0,95	0,94
	Lokal	0,98	0,97	0,96	0,94	0,93
Empat-lajur tak-terbagi:	Arteri	1,00	0,99	0,97	0,96	0,945
	Kolektor	0,97	0,96	0,94	0,93	0,915
	Lokal	0,95	0,94	0,92	0,91	0,895
Dua-lajur tak-terbagi	Arteri	1,00	0,98	0,97	0,96	0,94
	Kolektor	0,94	0,93	0,91	0,90	0,88
	Lokal	0,90	0,88	0,87	0,86	0,84

Sumber : MKJI (1997)

2.4 Peramalan Lalu Lintas

Untuk memperkirakan biaya – biaya yang akan dikeluarkan dimasa yang akan datang, seiring dengan bertambahnya jumlah kendaraan.

2.4.2 Persamaan Regresi Linear

Peramalan dengan metode regresi linier akan didapatkan persamaan garis linier dengan data yang telah ada. Terdapat hubungan fungsional antara variable – variabelnya dalam persamaan linier yang dihasilkan. Jumlah kendaraan dinyatakan sebagai variabel tidak bebas dengan notasi Y sedangkan tahun dinyatakan sebagai variabel bebas dengan notasi X . Secara Matematis dapat dirumuskan dalam persamaan:

Penyimpangan diusahakan sekecil mungkin agar sesuai dengan data yang ada, hal ini dinyatakan dalam bentuk matematis sebagai hubungan fungsional antara variabel – variabelnya secara matematis.

$$Y = ax + b$$

Sedangkan harga untuk koefisien a dan b dapat dicari dengan persamaan sebagai berikut :

$$a = \frac{(n \sum xy - \sum x \sum y)}{(n \sum x^2 - (\sum x)^2)} \dots\dots\dots \text{Pers. 2.6}$$

$$b = \frac{(\sum y - a * \sum x)}{n} \dots\dots\dots \text{Pers. 2.7}$$

$$r = \frac{(n \sum xy - \sum x \sum y)}{\sqrt{(n \sum x^2 - (\sum x)^2) (n \sum y^2 - (\sum y)^2)}} \dots\dots \text{Pers. 2.8}$$

Dimana :

- a dan b = koef regresi
- Y = variabel tidak bebas
- X = variabel bebas
- n = jumlah data

2.5 Biaya Operasi Kendaraan

Biaya yang digunakan kendaraan untuk beroperasi dari suatu tempat menuju ke tempat lain. Metode yang digunakan dalam tugas akhir ini menggunakan metode Jasa Marga. Biaya operasional kendaraan (BOK) terdiri dari biaya konsumsi bahan bakar, biaya konsumsi minyak pelumas, biaya pemakaian ban, biaya pemeliharaan, biaya penyusutan, bunga modal, dan biaya asuransi.

2.5.1 Biaya Konsumsi Bahan Bakar

Biaya yang dibutuhkan untuk konsumsi suku cadang kendaraan dalam pengoperasian suatu jenis kendaraan per kilometer jarak tempuh.

$$\mathbf{KBB} = \mathbf{KBB\ dasar} \times (\mathbf{1} \pm (\mathbf{k}_k + \mathbf{k}_l + \mathbf{k}_r)) \dots\dots \text{Pers .2.9}$$

KBB = Konsumsi bahan bakar

$$\text{KBB dasar kendaraan golongan I} = 0,0284 V^2 - 3,0644 V + 141,68$$

$$\text{KBB dasar kendaraan golongan IIA} = 2,26533 \times (\text{KBB dasar golongan I})$$

$$\text{KBB dasar kendaraan golongan IIB} = 2,90805 \times (\text{KBB dasar golongan I})$$

k_k = faktor koreksi akibat kelandaian

k_l = faktor koreksi akibat kondisi arus lalu lintas

k_r = faktor koreksi akibat kekasaran jalan

V = kecepatan kendaraan (km/jam)

Tabel 2.12 Faktor koreksi konsumsi bahan bakar dasar kendaraan (k_k)

Faktor koreksi akibat kelandaian negatif (k_k)	$g < -5\%$	-0,337
	$-5\% < g < 0\%$	-0,158
Faktor koreksi akibat kelandaian positif (k_k)	$0\% < g < 5\%$	0,400
	$g > 5\%$	0,820
Faktor koreksi akibat kondisi arus lalu lintas (k_l)	$0 < NVK < 0,6$	0,050
	$0,6 < NVK < 0,8$	0,185
	$NVK > 0,8$	0,253
Faktor koreksi akibat kekasaran jalan (k_r)	$< 3 \text{ m/km}$	0,035
	$> 3 \text{ m/km}$	0,085

g = kelandaian

NVK = nisbah volume per kapasitas

Sumber : Tamin (2000)

2.5.2 Biaya konsumsi oli

Besarnya konsumsi minyak pelumas (liter/km) sangat tergantung pada kecepatan kendaraan dan jenis kendaraan. Konsumsi dasar ini kemudian dikoreksi lagi menurut tingkat kekasaran jalan.

Tabel 2.13 Konsumsi dasar minyak pelumas (liter/km)

Kecepatan (km/jam)	Jenis kendaraan		
	Golongan I	Golongan IIA	Golongan IIB
10-20	0,0032	0,0060	0,0049
20-30	0,0030	0,0057	0,0046
30-40	0,0028	0,0055	0,0044
40-50	0,0027	0,0054	0,0043
50-60	0,0027	0,0054	0,0043
60-70	0,0029	0,0055	0,0044
70-80	0,0031	0,0057	0,0046
80-90	0,0033	0,0060	0,0049
90-100	0,0035	0,0064	0,0053
100-110	0,0038	0,0070	0,0059

Sumber : Tamin (2000)

Tabel 2.14 Faktor koreksi konsumsi minyak pelumas terhadap kondisi kekasaran permukaan

Nilai kekasaran	Faktor koreksi
< 3 m/km	1,00
> 3 m/km	1,50

Sumber : Tamin (2000)

2.5.3 Biaya konsumsi ban

Biaya pemakaian ban besarnya biaya pemakaian ban sangat tergantung pada kecepatan kendaraan dan jenis kendaraan.

Kendaraan golongan I : $Y = 0,0008848 V - 0,0045333$..Pers. 2.10

Kendaraan golongan IIA: $Y = 0,0012356 V - 0,0064667$..Pers. 2.11

Kendaraan golongan IIB: $Y = 0,0015553 V - 0,0059333$...Pers.2.12

Y = pemakaian ban per 1.000 km

2.5.4 Biaya pemeliharaan

Biaya pemeliharaan Komponen biaya pemeliharaan yang paling dominan adalah biaya suku cadang dan upah montir.

a Suku cadang

Golongan I : $Y = 0,0000064 V + 0,0005567$... Pers. 2.13

Golongan IIA : $Y = 0,0000332 V + 0,0020891$... Pers. 2.14

Golongan IIB : $Y = 0,0000191 V + 0,0015400$...Pers. 2.15

Y = biaya pemeliharaan suku cadang 5per 1.000 km

b Montir

Golongan I : $Y = 0,00362 V + 0,36267$ Pers. 2.16

Golongan IIA : $Y = 0,02311 V + 1,97733$ Pers. 2.17

Golongan IIB : $Y = 0,01511 V + 1,21200$ Pers. 2.18

Y = jam kerja montir per 1.000 km

2.5.5 Biaya penyusutan

Biaya penyusutan hanya berlaku untuk perhitungan BOK pada jalan tol dan jalan arteri, besarnya berbanding terbalik dengan kecepatan kendaraan.

Golongan I : $Y = 1/(2,5 V + 125)$ Pers. 2.19

Golongan IIA : $Y = 1/(9,0 V + 450)$ Pers. 2.20

Golongan IIB : $Y = 1/(6,0 V + 300)$ Pers. 2.21

Y = biaya penyusutan per 1.000 km (sama dengan 1/2 nilai penyusutan kendaraan)

2.5.6 Bunga modal

Bunga modal menurut *Road User Costs Model* (1991), besarnya biaya bunga modal per kendaraan per 1.000 km ditentukan oleh persamaan berikut.

Bunga modal = 0,22% x (harga kendaraan baru)..... Pers. 2.22

2.5.7 Biaya asuransi

Biaya asuransi besarnya biaya asuransi berbanding terbalik dengan kecepatan. Semakin tinggi kecepatan kendaraan, semakin kecil biaya asuransi.

Golongan I : $Y = 38/(500 V)$ Pers. 2.23

Golongan IIA : $Y = 6/(2571,42857 V)$ Pers. 2.24

Golongan IIB : $Y = 61/(1714,28571 V)$ Pers. 2.25

Y = biaya asuransi per 1.000 km

2.6 Nilai Waktu Kendaraan

Penghematan waktu perjalanan yang dinilai secara ekonomis untuk masing – masing pemakai jalan. Dan nilai waktu ialah sejumlah uang yang dikeluarkan seseorang untuk menghemat satu unit waktu perjalanan. Nilai waktu biasanya sebanding dengan pendapatan per kapita, merupakan perbandingan yang tetap dengan

tingkat pendapatan. Besarnya nilai waktu berbeda- beda menurut jenis kendaraan dan lokasi studi.

Nilai Waktu = maksimum { (k x nilai waktu dasar), nilai waktu minimum}Pers. 2.26

Beberapa modifikasi dilakukan untuk memilih nilai waktu yang terbesar antar nilai waktu dasar yang dikoreksi menurut lokasi dengan nilai waktu minimum. Dalam laporan ini didapat besarnya nilai waktu, nilai waktu dasar yang diambil nilai waktu dari berbagai studi pada tabel dibawah ini.

Tabel 2.15 Nilai waktu minimum (Rupiah/jam/kendaraan)

No	Kabupaten/ Kodya	Jasa Marga			JIUTR		
		Gol I	Gol IIA	Gol IIB	Gol I	Gol IIA	Gol IIB
1	DKI – Jakarta	8.200	12.369	9.188	8.200	17.022	4.246
2	Selain DKI - Jakarta	6.000	9.051	6.723	6.000	12.455	3.107

Sumber : Tamin (2000)

Tabel 2.16 Nilai Waktu Dasar Dari Berbagai Studi (Rp/Jam/Kend)

No	Referensi	Jasa Marga		
		Gol I	Gol IIa	Gol Iib
1	PT. Jasa Marga (1990-1996)	12287	18534	13768
2	Padalarang – Cileunyi (1996)	3385-5425	3827-38344	5716
3	Semarang (1996)	3411-6221	14541	1506
4	IHCM (1995)	3281	18212	4971
5	PCI (1979)	1341	3827	3152
6	JIUTR Northem Extension (PCI, 1989)	7076	14670	3659
7	Surabaya, Mojokerto (JICA, 1991)	8880	7960	7980

Sumber : Tamin (2000)

2.7 Analisa Ekonomi

Evaluasi ekonomi mencakup evaluasi kelayakan pembangunan jalan dengan memperhitungkan nilai – nilai sebagai berikut :

a. Benefit Cost Ratio (BCR)

Membandingkan semua manfaat (benefit) dengan biaya (cost) total yang dibutuhkan, setelah dikonversikan kedalam nilai uang sekarang (present value). Dengan perumusan :

$$B / C = \frac{\text{Benefit}}{\text{Cost}} \dots\dots\dots \text{Pers. 2.27}$$

Benefit = penghematan biaya operasi kendaraan, penghematan nilai waktu.

Cost = biaya pembangunan jalan dan biaya pemeliharaan.

Nilai B/C yang mungkin :

$B/C > 1$: Berarti manfaat yang ditimbulkan proyek lebih besar dari biaya yang diperlukan secara ekonomi, proyek layak untuk dilaksanakan.

$B/C = 1$: Berarti manfaat yang ditimbulkan proyek sama dengan biaya yang diperlukan secara ekonomi, proyek layak dilaksanakan.

$B/C < 1$: Berarti manfaat yang ditimbulkan proyek lebih kecil dari biaya yang diperlukan secara ekonomi proyek tidak layak untuk dilaksanakan.

b. Net Present Value (NPV)

Mengurangi semua manfaat (benefit) dengan biaya (cost) total yang dibutuhkan setelah di konversikan ke dalam nilai uang sekarang. Dengan perumusan :

$$NPV = \text{Benefit} - \text{Cost} \dots\dots\dots \text{Pers. 2.28}$$

Suatu proyek bisa dikatakan layak jika $NPV > 0$, yaitu bila manfaat yang dihasilkan proyek lebih besar dari biaya yang diperlukan untuk pelaksanaan pekerjaan.

“Halaman ini sengaja dikosongkan”

BAB III METODOLOGI

3.1 Umum

Sebelum mengerjakan tugas akhir ini maka perlu disusun langkah-langkah pengerjaan sesuai dengan uraian kegiatan yang akan dilakukan dan dibuat diagram alir metodologi.

3.2 Uraian Kegiatan

- **Studi literatur dan bahan pustaka**

Dalam penyusunan tugas akhir ini di perlukan literatur yang digunakan sebagai acuan untuk menyusun tugas akhir ini .Literatur yang di gunakan :

1. Manual Kapasitas Jalan Indonesia (MKJI)
2. Perencanaan dan Pemodelan Transportasi Ofyar Z.Tamin.

- **Penentuan lokasi survey traffic counting**

Lokasi survey berada pada bagian tertentu ruas jalan antara batas kota Tuban sampai kecamatan Bulu.

- **Pengumpulan data sekunder.**

Data sekunder adalah data yang diperoleh melalui instansi terkait atau dari studi yang telah ada, yang terdiri dari:

1. Data Tata guna lahan di daerah sekitar
2. Data geometrik jalan
3. Data LHRT (Lalu-Lintas Harian Rata-rata Tahunan)

- **Pengolahan data**

1. Peramalan Pertumbuhan Jumlah Kendaraan (Forecasting)
Peramalan menggunakan regresi linier dengan memasukkan data volume lalu lintas harian rata – rata (LHRT) untuk mendapatkan volume kendaraan pada tahun rencana.

2. Analisa lalu lintas

Dilakukan untuk meninjau kondisi lalu lintas eksisting maupun ataupun rencana, Data diperoleh dari melakukan *survey traffic counting* pada lokasi studi dan untuk mempermudah perhitungan maka digunakan program bantu KAJI sesuai dengan MKJI 1997, untuk mendapatkan derajat kejenuhan ,kecepatan,dan waktu tempuh.

3. Menghitung biaya operasional kendaraan

Dengan menggunakan data kecepatan kendaraan eksisting dan rencana yang diperoleh dari program bantu KAJI maka dengan menggunakan metode Jasa Marga didapatkan biaya operasional perkendaraan baik itu dengan kecepatan kondisi eksisting maupun rencana.

4. Menghitung biaya nilai waktu

Dengan menggunakan data yang telah terkumpul yaitu nilai waktu dan jumlah kendaraan/hari. Maka bisa didapatkan biaya nilai waktu kondisi eksisting maupun rencana.

• Analisa ekonomi

1. Penghematan (BOK) Biaya Operasional Kendaraan

Penghematan biaya operasional kendaraan merupakan selisih antara biaya operasional kendaraan pada jalan eksisting dan biaya operasional kendaraan setelah dilakukan pelebaran jalan.

2. Penghematan Nilai Waktu

Perhitungan penghematan nilai waktu diperoleh dengan membandingkan nilai waktu pada kondisi eksisting dan kondisi setelah dilebarkan.

3. Benefit Cost Ratio

Membandingkan antara besarnya biaya penghematan (benefit) dengan besarnya biaya pembangunan (cost). Untuk memenuhi syarat layak

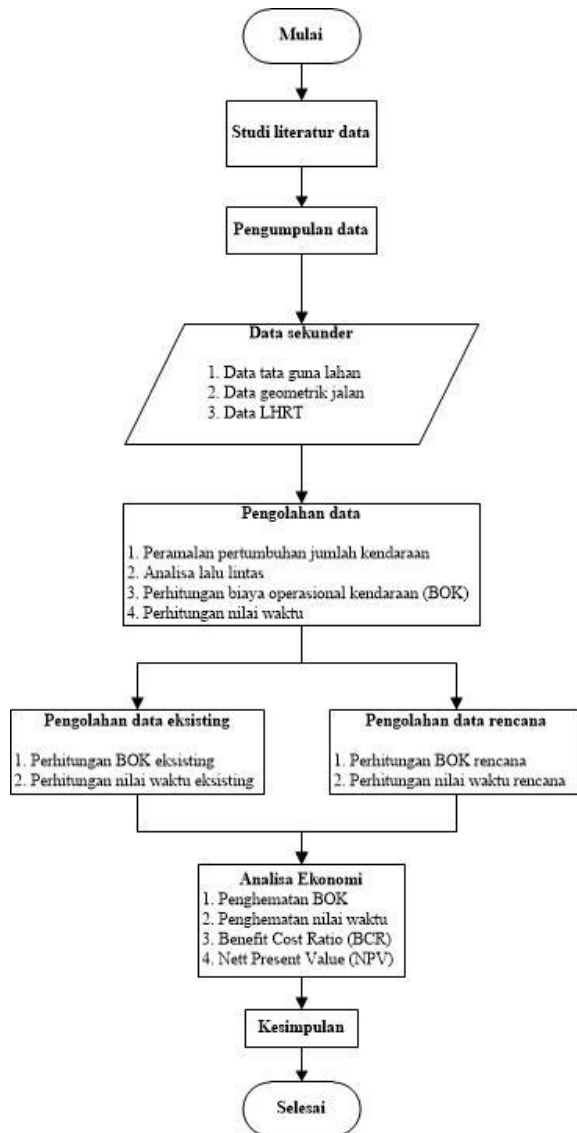
tidaknya pelebaran jalan ini dilaksanakan nilai BCR harus lebih besar dari 1 ($BCR > 1$).

4. Net Present Value

Selisih antara pengeluaran dan pemasukan. Pelebaran jalan di jalan bulu Tuban layak dilaksanakan bila nilai NPV > 0 .

3.3 Bagan Alir Metodologi

Tahapan pengerjaan proposal tugas akhir ini dapat ditunjukkan dalam bagan alir seperti pada gambar 3.1



Gambar 3.1 Bagan Alir

BAB IV DATA DAN ANALISA

4.1 Umum

Analisa data diperoleh dengan cara mengolah data sekunder. Kemudian dari pengolahan data tersebut dilanjutkan dengan analisa ekonomi yang meliputi biaya operasional kendaraan (BOK), nilai waktu, benefit cost ratio (BCR), dan net present value (NPV).

4.2 Peramalan

Data jumlah kendaraan bermotor dari tahun 2007 sampai tahun 2010 digunakan untuk meramalkan pertumbuhan lalu lintas untuk masing masing jenis kendaraan .Untuk mengolah data pertumbuhan jumlah kendaraan tiap tahun digunakan program bantu (MS Excel) yaitu dengan cara perhitungan regresi linear. Langkah langkah dalam mencari pertumbuhan jumlah kendaraan adalah sebagai berikut :

1. Dari data masing masing jumlah kendaraan bermotor dapat diperoleh grafik dan persamaan regresi.
2. Cek grafik regresi apabila nilai r mendekati 1 maka rumus dari grafik tersebut mendekati data yang sebenarnya.
3. Dari persamaan regresi dapat diperoleh prediksi pertumbuhan kendaraan untuk masing masing tahun pada umur 20 tahun mendatang dengan menggunakan rumus hasil regresi.
4. Dari hasil perhitungan persamaan regresi dapat diperoleh pertumbuhan tiap kendaraan untuk masing masing tahun.

5. Dengan jumlah hasil dari hitungan persamaan pertumbuhan lalu lintas pada tiap kendaraan untuk masing masing tahun dapat diperoleh rata rata pertumbuhan lalu lintas (i).
6. Kemudian kita ubah hasil dari rata rata pertumbuhan lalu lintas (i) kedalam bentuk persen (%).maka dapat diketahui pertumbuhan lalu lintas dari masing masing jenis kendaraan.

Tabel 4.1 Data jumlah kendaraan jalan Bulu – Tuban tahun 2007 – 2010

Jenis Kendaraan	Tahun			
	2007	2008	2009	2010
MC	3720	4322	4957	5452
LV	3345	3891	4568	5291
MHV	414	548	650	744
LB	14	15	21	24
LT	311	401	446	559

(sumber : Dinas Bina Marga Provinsi Jawa Timur)

Hasil regresi dari data jumlah masing masing kendaraan dari tahun 2007 – 2010 dapat diperoleh pertumbuhan lalu lintas untuk masing masing jenis kendaraan sebagai berikut :

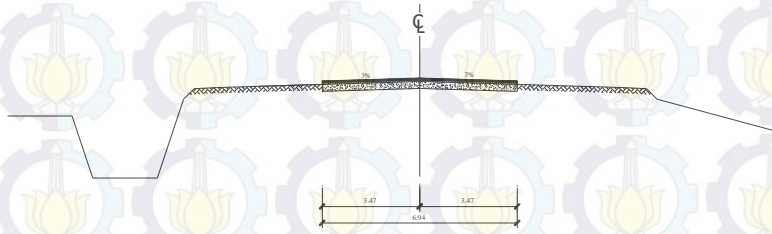
Tabel 4.2 Data pertumbuhan jumlah kendaraan jalan Bulu – Tuban.

No	Jenis Kendaraan	Pertumbuhan Kendaraan (%)
1	MC	6.36
2	LV	6.97
3	MHV	7.43
4	LB	8.36
5	LT	8.34

4.3 Kondisi Eksisting Ruas Jalan

Kondisi ruas jalan Bulu – Tuban saat ini adalah berupa jalan arteri primer yang mana merupakan jalan nasional yang menghubungkan antar kota. jalan tersebut berfungsi melayani angkutan utama dengan ciri ciri perjalanan jarak jauh, kecepatan rata rata tinggi, dan jumlah jalan masuk dibatasi secara daya guna. Analisa kondisi eksisting pada jalan tersebut dianalisa sampai 20 tahun mendatang yaitu sampai tahun 2034 dengan pertimbangan volume jalan raya pada tahun tersebut yang semakin padat, sehingga untuk menghindari kemacetan tersebut diperlukan pelebaran jalan dari 2 lajur 2 arah tak terbagi menjadi 4 lajur 2 arah tak terbagi. karakteristik ruas jalan Bulu – Tuban adalah sebagai berikut:

- Tipe jalan : 2 lajur 2 arah tak terbagi (2/2 UD)
- Lebar jalan : 7m
- Panjang jalan : 7,5 km



Gambar 4.1 Potongan melintang jalan Bulu Tuban eksisting.

Dari hasil peramalan volume kendaraan maka dapat dicari tingkat kinerja jalan eksisting pada tahun 2014 sampai dengan tahun 2034 dengan menggunakan program bantu (KAJI) .

Tabel 4.3 Perkiraan volume LHR (kend/hari) masing masing jenis kendaraan

Tahun	LV	MHV	LB	LT	MC	TOTAL
2014	7856	8699	38	864	7820	25277
2015	8508	9365	42	943	8403	27261
2016	9159	10032	46	1022	8986	29244
2017	9810	10699	49	1101	9569	31228
2018	10462	11366	53	1180	10152	33212
2019	11113	12033	56	1258	10735	35196
2020	11765	12699	60	1337	11318	37180
2021	12416	13366	64	1416	11902	39164
2022	13068	14033	67	1495	12485	41148
2023	13719	14700	71	1574	13068	43132
2024	14371	15367	74	1653	13651	45116
2025	15023	16033	78	1732	14234	47100
2026	15674	16700	82	1811	14817	49083
2027	16325	17367	85	1890	15400	51067
2028	16977	18034	89	1969	15983	53051
2029	17628	18701	92	2047	16566	55035
2030	18280	19367	96	2126	17149	57019
2031	18931	20034	100	2205	17733	59003
2032	19583	20701	103	2284	18316	60987
2033	20234	21368	107	2363	18899	62971
2034	20886	22035	110	2442	19482	64955

Hasil dari analisa program KAJI akan diperoleh nilai derajat kejenuhan (DS) ,waktu tempuh (travel time),dan kecepatan aktual kendaraan.

Contoh perhitungan kecepatan aktual dan travel time masing-masing jenis kendaraan pada kondisi eksisting tahun 2014 adalah sebagai berikut :

Data jalan eksisting

Tipe jalan : 2 lajur 2 arah tak terbagi (2/2 UD)
 Kelas jalan : Jalan nasional
 Kelas fungsional : Arteri
 Panjang jalan : 7,5 km
 Tipe alinyemen : Datar

Penampang melintang jalan:

Lebar lalu lintas rata-rata (W_c) : 3,5 m
 Lebar bahu efektif (W_s) : 3,7 m

Kondisi permukaan jalan:

Tipe Perkerasan : Perkerasan lentur
 Kondisi bahu :
 Tipe permukaan:
 Beda tinggi :

Lalu lintas harian rata rata tahunan:

emp:

LV = 1 ; MHV = 1,3 ; LB=1,5 ; LT = 2,5 ; MC = 0,5
 k = 0,08

data jumlah masing-masing kendaraan (kend/hari)

Tahun	LV	MHV	LB	LT	MC	TOTAL
2014	7856	8699	38	864	7820	25277

$$\begin{aligned} LV &= \text{Volume LV(kend/hari) x k} \\ &= 7856 \times 0,08 = 628 \text{ kend/jam} \end{aligned}$$

$$\begin{aligned} MHV &= \text{Volume MHV(kend/hari) x k} \\ &= 8699 \times 0,08 = 696 \text{ kend/jam} \end{aligned}$$

$$\begin{aligned} LB &= \text{Volume LB(kend/hari) x k} \\ &= 38 \times 0,08 = 3 \text{ kend/jam} \end{aligned}$$

$$\begin{aligned} LT &= \text{Volume LT(kend/hari) x k} \\ &= 864 \times 0,08 = 69 \text{ kend/jam} \end{aligned}$$

$$\begin{aligned} MC &= \text{Volume MC(kend/hari) x k} \\ &= 7820 \times 0,08 = 626 \text{ kend/jam} \end{aligned}$$

$$\begin{aligned} LV &= \text{Volume kend/hari x k x emp} \\ &= 628 \times 1 = 628 \text{ smp/jam} \end{aligned}$$

$$\begin{aligned} MHV &= \text{Volume kend/hari x k x emp} \\ &= 696 \times 1,3 = 905 \text{ smp/jam} \end{aligned}$$

$$\begin{aligned} LB &= \text{Volume kend/hari x k x emp} \\ &= 3 \times 1,5 = 5 \text{ smp/jam} \end{aligned}$$

$$\begin{aligned} LT &= \text{Volume kend/hari x k x emp} \\ &= 69 \times 2,5 = 173 \text{ smp/jam} \end{aligned}$$

$$\begin{aligned} MC &= \text{Volume kend/hari x k x emp} \\ &= 626 \times 0,5 = 313 \text{ smp/jam} \end{aligned}$$

Arus total (Q)

$$\begin{aligned} Q &= Q_{LV} + Q_{MHV} + Q_{LB} + Q_{LT} + Q_{MC} \\ &= 628 + 696 + 3 + 69 + 626 = 2022 \text{ kend/jam} \end{aligned}$$

$$\begin{aligned} Q &= Q_{LV} + Q_{MHV} + Q_{LB} + Q_{LT} + Q_{MC} \\ &= 628 + 905 + 5 + 173 + 313 = 2023 \text{ smp/jam} \end{aligned}$$

Hambatan sampling = L

Kecepatan arus bebas kendaraan ringan

$$F_{vo} = 68 \text{ km/jam}$$

$$F_{vw} = 0 \text{ km/jam}$$

$$F_{vo} + F_{vw} = 68 + 0 = 68 \text{ km/jam}$$

$$FF_{vsf} = 0,98$$

$$FF_{vcs} = 1$$

$$FV = (F_{vo} + F_{vw}) \times FF_{vsf} \times FF_{vcs}$$

$$= 68 \times 0,98 \times 1 = 66,64 \text{ km/jam}$$

Kecepatan arus bebas kendaraan kendaraan berat menengah (MHV)

$$FFV = F_{Vo} - FV$$

$$= 68 - 66,64 = 1,36 \text{ km/jam}$$

$$FV_{MHV} = FV_{MHV0} - FFV \times FV_{MHV0} / F_{Vo}$$

$$= 60 - 1,36 \times 60 / 68 = 58,8 \text{ km/jam}$$

Kecepatan arus bebas kendaraan kendaraan berat (LT)

$$FFV = F_{Vo} - FV$$

$$= 68 - 66,64 = 1,36 \text{ km/jam}$$

$$FV_{LT} = FV_{LT0} - FFV \times FV_{LT0} / F_{Vo}$$

$$= 58 - 1,36 \times 58 / 68 = 56,84 \text{ km/jam}$$

Kapasitas

$$C_o = 3100 \text{ smp/jam}$$

$$FC_w = 1$$

$$FC_{SP} = 0,97$$

$$FC_{SF} = 1$$

$$C = C_o \times FC_w \times FC_{SP} \times FC_{SF}$$

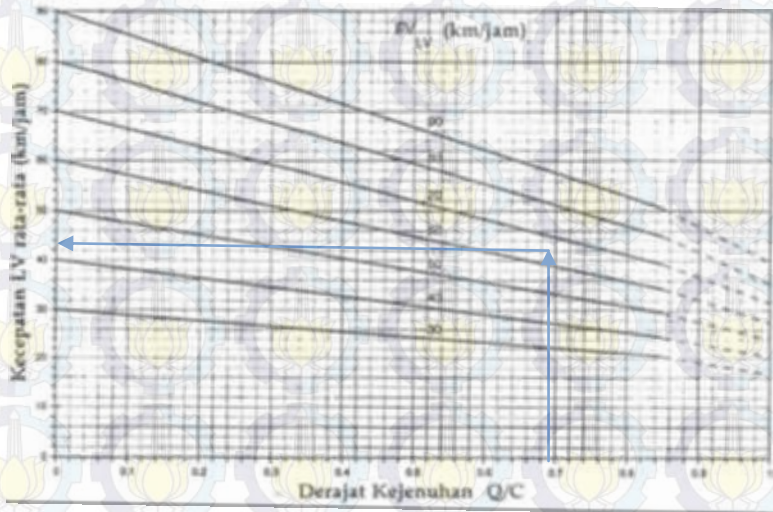
$$= 3100 \times 1 \times 0,97 \times 1 = 3007 \text{ smp/jam}$$

Kecepatan kendaraan ringan

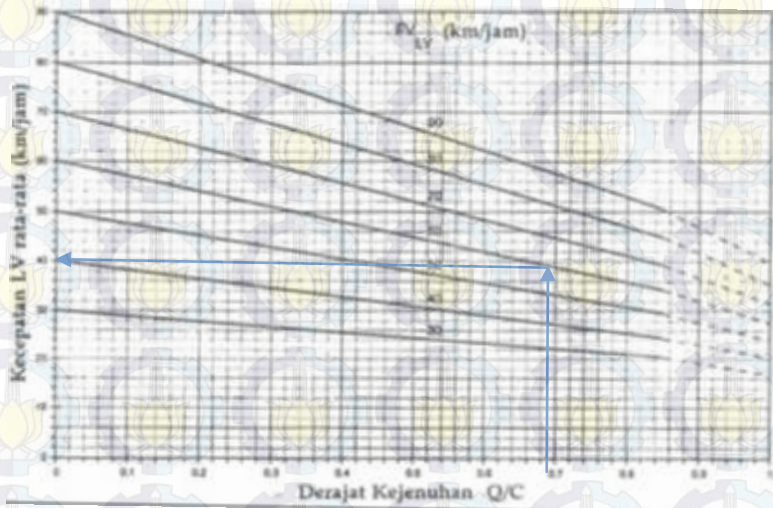
$$Q = 2023 \text{ smp/jam}$$

$$DS = Q/C$$

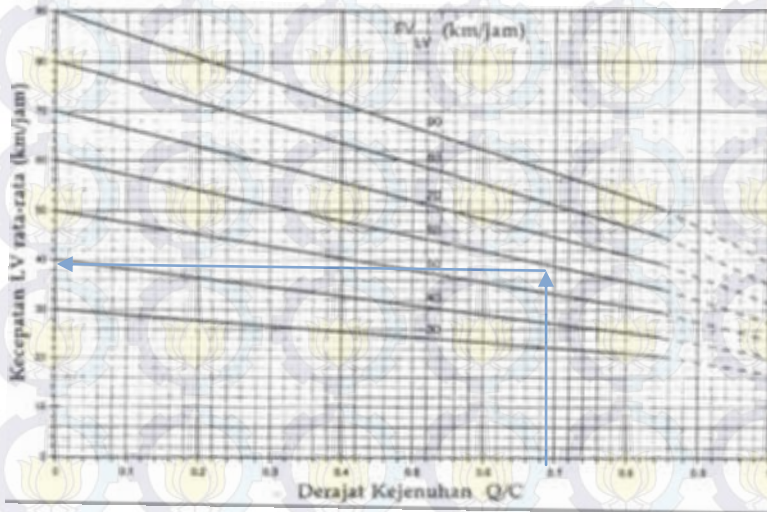
$$= 2023/3007 = 0,673$$



Gambar 4.2 Grafik hubungan antara DS dan kecepatan LV



Gambar 4.3 Grafik hubungan antara DS dan kecepatan MHV



Gambar 4.4 Grafik hubungan antara DS dan kecepatan LT

$$V_{LV} = 43,58 \text{ km/jam}$$

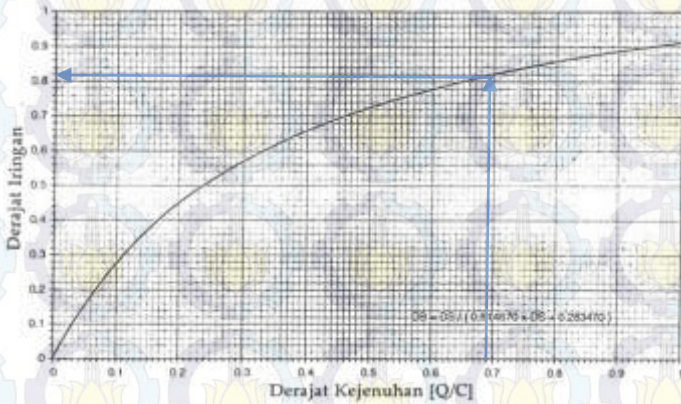
$$V_{MHV} = 40,70 \text{ km/jam}$$

$$V_{LT} = 39,98 \text{ km/jam}$$

$$\begin{aligned} TT_{LV} &= \text{Panjang jalan} / V_{LV} \times 3600 \\ &= 7,5 / 42,47 \times 3600 = 635,743 \text{ detik} \end{aligned}$$

$$\begin{aligned} TT_{MHV} &= \text{Panjang jalan} / V_{MHV} \times 3600 \\ &= 7,5 / 42,47 \times 3600 = 635,743 \text{ detik} \end{aligned}$$

$$\begin{aligned} TT_{LT} &= \text{Panjang jalan} / V_{LT} \times 3600 \\ &= 7,5 / 42,47 \times 3600 = 635,743 \text{ detik} \end{aligned}$$

Derajat iringan (DB)**Gambar 4.5** Grafik hubungan antara DS dan derajat iringan

$$DB = 0,81$$

Tabel 4.4 Tabel derajat kejenuhan, kecepatan ,dan traveltime untuk kondisi jalan eksisting kendaraan gol I.

TAHUN	DS	KECEPATAN AKTUAL GOL I (KM/JAM)	TRAVEL TIME (DETIK)
2014	0.673	43.58	619.433
2015	0.749	40.58	665.352
2016	0.804	38.74	696.954
2017	0.860	36.70	735.695
2018	0.914	34.10	791.789
2019	0.969	31.41	859.599
2020	1.025	N/A	N/A
2021	1.081	N/A	N/A
2022	1.135	N/A	N/A
2023	1.191	N/A	N/A
2024	1.246	N/A	N/A
2025	1.302	N/A	N/A
2026	1.356	N/A	N/A
2027	1.411	N/A	N/A
2028	1.467	N/A	N/A
2029	1.522	N/A	N/A
2030	1.578	N/A	N/A
2031	1.631	N/A	N/A
2032	1.687	N/A	N/A
2033	1.742	N/A	N/A
2034	1.797	N/A	N/A

Tabel 4.5 Tabel kecepatan ,dan traveltime untuk kondisi jalan eksisting kendaraan gol II A.

TAHUN	KECEPATAN AKTUAL GOL II A (KM/JAM)	TRAVEL TIME (DETIK)
2014	40.7	663.3906634
2015	38.28	705.3291536
2016	36.84	732.8990228
2017	35.24	766.1748014
2018	33.2	813.253012
2019	31.09	868.4464458
2020	N/A	N/A
2021	N/A	N/A
2022	N/A	N/A
2023	N/A	N/A
2024	N/A	N/A
2025	N/A	N/A
2026	N/A	N/A
2027	N/A	N/A
2028	N/A	N/A
2029	N/A	N/A
2030	N/A	N/A
2031	N/A	N/A
2032	N/A	N/A
2033	N/A	N/A
2034	N/A	N/A

Tabel 4.6 Tabel kecepatan ,dan traveltime untuk kondisi jalan eksisting kendaraan gol II B.

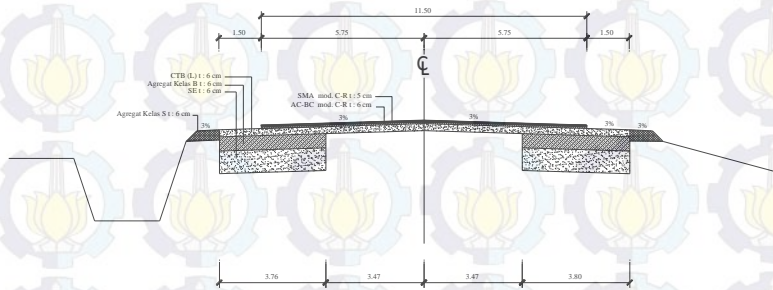
TAHUN	KECEPATAN AKTUAL GOL IIB (KM/JAM)	TRAVEL TIME (DETIK)
2014	39.98	675.337669
2015	37.71	715.990453
2016	36.37	742.370085
2017	34.88	774.082569
2018	32.97	818.926297
2019	31.01	870.686875
2020	N/A	N/A
2021	N/A	N/A
2022	N/A	N/A
2023	N/A	N/A
2024	N/A	N/A
2025	N/A	N/A
2026	N/A	N/A
2027	N/A	N/A
2028	N/A	N/A
2029	N/A	N/A
2030	N/A	N/A
2031	N/A	N/A
2032	N/A	N/A
2033	N/A	N/A
2034	N/A	N/A

Tabel 4.7 Tabel derajat iringan kendaraan

TAHUN	DB
2014	0.81
2015	0.84
2016	0.86
2017	0.87
2018	0.89
2019	0.90
2020	N/A
2021	N/A
2022	N/A
2023	N/A
2024	N/A
2025	N/A
2026	N/A
2027	N/A
2028	N/A
2029	N/A
2030	N/A
2031	N/A
2032	N/A
2033	N/A
2034	N/A

4.4 Kondisi Rencana Ruas Jalan

Pada kondisi jalan rencana, jalan bulu - tuban akan diperlebar 3.5 m pada masing masing arah sehingga jalan tersebut menjadi 4 lajur 2 arah. Setelah dilakukan pelebaran, jalan tersebut dianalisa sampai dengan 20 tahun mendatang yaitu tahun 2034. Untuk mengetahui kinerja jalan tersebut, dengan menggunakan program bantu (KAJI) maka dapat diperoleh nilai derajat kejenuhan (DS), waktu tempuh (travel time), dan kecepatan aktual kendaraan pada jalan rencana.



Gambar 4.6 Potongan melintang jalan Bulu Tuban rencana

Data jalan rencana

Tipe jalan : 4 lajur 2 arah tak terbagi (4/2 UD)
 Kelas jalan : Jalan nasional
 Kelas fungsional : Arteri
 Panjang jalan : 7,5 km
 Tipe alinyemen : Datar

Penampang melintang jalan:

Lebar lalu lintas rata-rata (Wc) : 5,75 m
 Lebar bahu efektif (Ws) : 1,5 m

Kondisi permukaan jalan:

Tipe Perkerasan : Perkerasan lentur
 Kondisi bahu :
 Tipe permukaan:
 Beda tinggi :

Lalu lintas harian rata rata tahunan:

emp:

LV = 1 ; MHV = 1,44 ; LB=1,46 ; LT = 2,1 ; MC = 0,64
 k = 0,08

data jumlah masing-masing kendaraan (kend/hari)

Tahun	LV	MHV	LB	LT	MC	TOTAL
2014	7856	8699	38	864	7820	25277

$$\begin{aligned} LV &= \text{Volume LV(kend/hari) } \times k \\ &= 7856 \times 0,08 = 628 \text{ kend/jam} \end{aligned}$$

$$\begin{aligned} MHV &= \text{Volume MHV(kend/hari) } \times k \\ &= 8699 \times 0,08 = 696 \text{ kend/jam} \end{aligned}$$

$$\begin{aligned} \text{LB} &= \text{Volume LB (kend/hari)} \times k \\ &= 38 \times 0,08 = 3 \text{ kend/jam} \end{aligned}$$

$$\begin{aligned} \text{LT} &= \text{Volume LT (kend/hari)} \times k \\ &= 864 \times 0,08 = 69 \text{ kend/jam} \end{aligned}$$

$$\begin{aligned} \text{MC} &= \text{Volume MC (kend/hari)} \times k \\ &= 7820 \times 0,08 = 626 \text{ kend/jam} \end{aligned}$$

$$\begin{aligned} \text{LV} &= \text{Volume kend/hari} \times k \times \text{emp} \\ &= 628 \times 1 = 628 \text{ smp/jam} \end{aligned}$$

$$\begin{aligned} \text{MHV} &= \text{Volume kend/hari} \times k \times \text{emp} \\ &= 696 \times 1,44 = 1002 \text{ smp/jam} \end{aligned}$$

$$\begin{aligned} \text{LB} &= \text{Volume kend/hari} \times k \times \text{emp} \\ &= 3 \times 1,46 = 4 \text{ smp/jam} \end{aligned}$$

$$\begin{aligned} \text{LT} &= \text{Volume kend/hari} \times k \times \text{emp} \\ &= 69 \times 2,1 = 145 \text{ smp/jam} \end{aligned}$$

$$\begin{aligned} \text{MC} &= \text{Volume kend/hari} \times k \times \text{emp} \\ &= 626 \times 0,64 = 401 \text{ smp/jam} \end{aligned}$$

Arus total (Q)

$$\begin{aligned} Q &= Q_{LV} + Q_{MHV} + Q_{LB} + Q_{LT} + Q_{MC} \\ &= 628 + 696 + 3 + 69 + 626 = 2022 \text{ kend/jam} \end{aligned}$$

$$\begin{aligned} Q &= Q_{LV} + Q_{MHV} + Q_{LB} + Q_{LT} + Q_{MC} \\ &= 628 + 1002 + 4 + 145 + 401 = 2180 \text{ smp/jam} \end{aligned}$$

$$\text{Hambatan samping} = L$$

Kecepatan arus bebas kendaraan ringan

$$F_{vo} = 74 \text{ km/jam}$$

$$F_{vw} = -3 \text{ km/jam}$$

$$F_{vo} + F_{vw} = 74 - 3 = 71 \text{ km/jam}$$

$$FF_{vsf} = 0,97$$

$$FF_{vcs} = 1$$

$$FV = (FV_o + FV_w) \times FF_{vsf} \times FF_{vcs}$$

$$= 71 \times 0,97 \times 1 = 68,87 \text{ km/jam}$$

Kecepatan arus bebas kendaraan kendaraan berat menengah (MHV)

$$FFV = FV_o - FV$$

$$= 74 - 68,87 = 5,13 \text{ km/jam}$$

$$FV_{MHV} = FV_{MHV_o} - FFV \times FV_{MHV_o} / FV_o$$

$$= 63 - 5,13 \times 63 / 74 = 58,63 \text{ km/jam}$$

Kecepatan arus bebas kendaraan kendaraan berat (LT)

$$FFV = FV_o - FV$$

$$= 74 - 68,87 = 5,13 \text{ km/jam}$$

$$FV_{LT} = FV_{LT_o} - FFV \times FV_{LT_o} / FV_o$$

$$= 60 - 5,13 \times 60 / 74 = 55,84 \text{ km/jam}$$

Kapasitas

$$C_o = 6800 \text{ smp/jam}$$

$$FC_w = 0,91$$

$$FC_{SP} = 0,975$$

$$FC_{SF} = 0,97$$

$$C = C_o \times FC_w \times FC_{SP} \times FC_{SF}$$

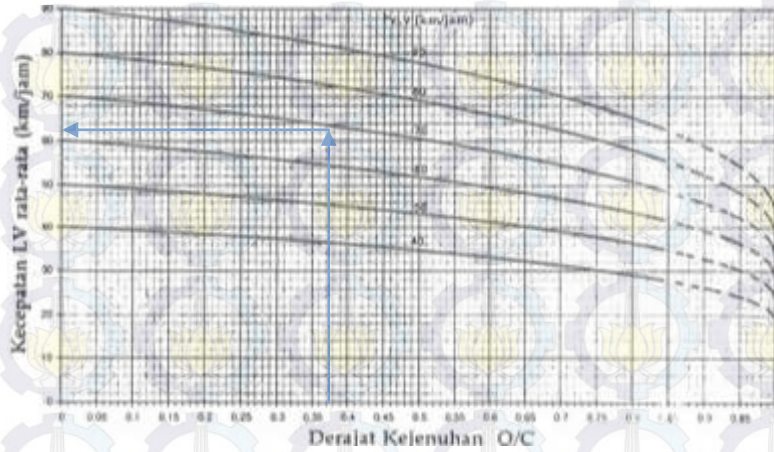
$$= 6800 \times 0,91 \times 0,975 \times 0,97 = 5852,301 \text{ smp/jam}$$

Kecepatan kendaraan ringan

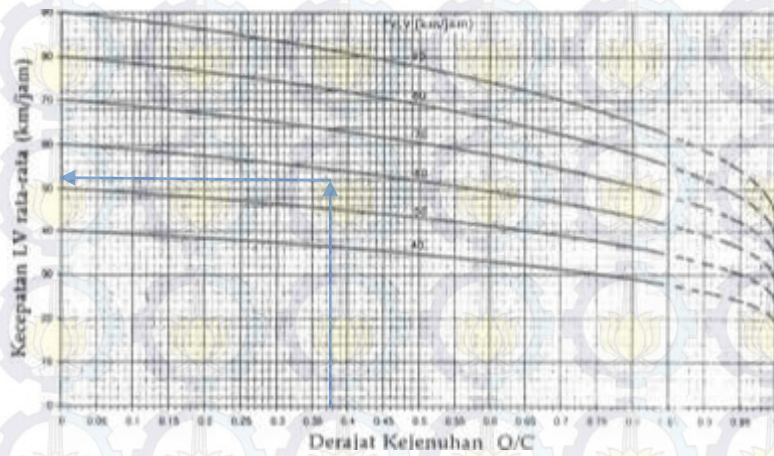
$$Q = 2180 \text{ smp/jam}$$

$$DS = Q/C$$

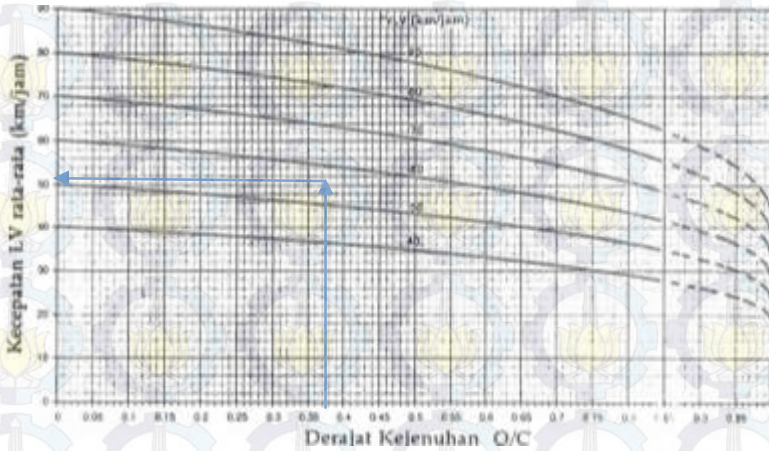
$$= 2023 / 5852,301 = 0,373$$



Gambar 4.7 Grafik hubungan antara DS dan kecepatan LV



Gambar 4.8 Grafik hubungan antara DS dan kecepatan MHV



Gambar 4.9 Grafik hubungan antara DS dan kecepatan LT

$$V_{LV} = 62,74 \text{ km/jam}$$

$$V_{MHV} = 53,41 \text{ km/jam}$$

$$V_{LT} = 50,87 \text{ km/jam}$$

$$\begin{aligned} TT_{LV} &= \text{Panjang jalan} / V_{LV} \times 3600 \\ &= 7,5 / 62,74 \times 3600 = 430,347 \text{ detik} \end{aligned}$$

$$\begin{aligned} TT_{MHV} &= \text{Panjang jalan} / V_{MHV} \times 3600 \\ &= 7,5 / 53,41 \times 3600 = 505,523 \text{ detik} \end{aligned}$$

$$\begin{aligned} TT_{LT} &= \text{Panjang jalan} / V_{LT} \times 3600 \\ &= 7,5 / 50,87 \times 3600 = 530,765 \text{ detik} \end{aligned}$$

Tabel 4.8 Tabel derajat kejenuhan, kecepatan ,dan traveltime untuk kondisi jalan rencana kendaraan gol I.

TAHUN	DS	KECEPATAN AKTUAL GOL I (KM/JAM)	TRAVEL TIME (DETIK)
2014	0.373	62.74	430.309
2015	0.408	61.97	435.675
2016	0.444	61.16	441.426
2017	0.481	60.27	447.939
2018	0.518	59.34	454.946
2019	0.556	58.34	462.773
2020	0.595	57.26	471.514
2021	0.636	56.07	481.516
2022	0.667	55.11	489.890
2023	0.671	54.97	491.102
2024	0.673	54.92	491.546
2025	0.672	54.96	491.218
2026	0.668	55.08	490.178
2027	0.690	54.35	496.719
2028	0.718	53.43	505.329
2029	0.745	52.47	514.559
2030	0.772	51.44	524.793
2031	0.798	50.39	535.81
2032	0.825	49.21	548.595
2033	0.852	47.96	562.915
2034	0.879	46.58	579.611

Tabel 4.9 Tabel kecepatan ,dan traveltime untuk kondisi jalan rencana kendaraan gol II A.

TAHUN	KECEPATAN AKTUAL GOL II A (KM/JAM)	TRAVEL TIME (DETIK)
2014	53.41	505.5233102
2015	52.760	511.7513268
2016	52.070	518.5327444
2017	51.310	526.2132138
2018	50.520	534.4418052
2019	49.670	543.5876787
2020	48.750	553.8461538
2021	47.730	565.681961
2022	46.920	575.4475703
2023	46.800	576.9230769
2024	46.760	577.4165954
2025	46.790	577.0463774
2026	46.89	575.815739
2027	46.27	583.5314459
2028	45.48	593.6675462
2029	44.67	604.432505
2030	43.8	616.4383562
2031	42.9	629.3706294
2032	41.9	644.3914081
2033	40.83	661.2784717
2034	39.65	680.9583859

Tabel 4.10 Tabel kecepatan ,dan traveltime untuk kondisi jalan rencana kendaraan gol II B.

TAHUN	KECEPATAN AKTUAL GOL IIB (KM/JAM)	TRAVEL TIME (DETIK)
2014	50.87	530.765
2015	50.240	537.420
2016	49.590	544.465
2017	48.570	555.899
2018	48.110	561.214
2019	47.300	570.825
2020	46.420	581.646
2021	45.460	593.929
2022	44.680	604.297
2023	44.570	605.789
2024	44.530	606.333
2025	44.560	605.925
2026	44.66	604.568
2027	44.07	612.662
2028	43.32	623.269
2029	42.54	634.697
2030	41.71	647.327
2031	40.85	660.955
2032	39.9	676.692
2033	38.89	694.266
2034	37.76	715.042

4.5 Analisa Ekonomi

Analisa ekonomi dilakukan untuk mengetahui kelayakan pelebaran jalan ditinjau dari segi ekonomi, yaitu meliputi biaya operasional kendaraan, nilai waktu, benefit cost ratio, dan net present value.

4.5.1 Biaya Operasional Kendaraan (BOK)

Biaya operasional kendaraan adalah penjumlahan dari biaya gerak (running cost) dan biaya tetap (standing cost). Metode perhitungan biaya operasional kendaraan yang digunakan dalam tugas akhir ini adalah dengan menggunakan metode jasa margas.

Penghematan biaya operasional kendaraan diperoleh dengan cara membandingkan biaya operasional kendaraan pada saat ruas jalan sebelum dilakukan pelebaran (eksisting) dengan pada saat ruas jalan telah dilakukan pelebaran. Parameter yang digunakan dalam perhitungan biaya operasional kendaraan yaitu kecepatan kendaraan yang diperoleh dari hasil program Kaji.

Selain kecepatan, parameter yang dibutuhkan untuk menghitung biaya operasional kendaraan adalah harga komponen dari masing-masing jenis kendaraan. Harga komponen masing-masing kendaraan adalah sebagai berikut:

1. Kendaraan Pribadi
 - Honda Mobilio RS type MT :Rp. 190.800.000
(Sumber : www.honda-indonesia.com)
 - BBM :Rp.8.500/Liter
 - Oli :Rp.49.500/Liter
 - Ban :Rp.1.100.000/Buah
 - Upah Montir :Rp.7.500/Jam
2. Bus
 - Bus sedang
Hino A215 Chasis+karoseri :Rp.740.300.000
(Sumber : www.e-hargamobil.com)
 - Bus besar
Hino R260 Chasis + Karoseri :Rp.1.212.200.000

(Sumber : www.e-hargamobil.com)

- BBM :Rp.7.500/Liter
- Oli :Rp.49.500/Liter
- Ban bus sedang :Rp.1.158.000/Buah
- Ban bus besar :Rp.1.432.000/Buah
- Upah Montir :Rp.7.500/Jam

3. Truk

- Truk tunggal berat
Hino FM 320 PD 10 Roda :Rp.1.001.000.000

(Sumber : www.e-hargamobil.com)

- Semi trailer
HINO FM320P + Ekor Trailer :Rp.1.118.700.000

(Sumber : www.e-hargamobil.com)

- BBM :Rp.7.500/Liter
- Oli :Rp. 49.500/Liter
- Ban truk tunggal berat :Rp. 1.432.000 /Buah
- Ban semi trailer :Rp. 1.446.000 /Buah
- Upah Montir :Rp. 7.500/Jam

Perhitungan besarnya BOK tiap kendaraan per 1000 km dari berbagai macam kecepatan yaitu dengan cara memasukan harga dari masing – masing komponen dari tiap jenis kendaraan dan dari persamaan BOK yang akan digunakan, maka dapat dicari berapa besar BOK.

Contoh hasil perhitungan BOK eksisting dengan menggunakan metode jasa marga adalah sebagai berikut :

Golongan I

1. Konsumsi Bahan Bakar

Konsumsi BBM = konsumsi BBM dasar x $(1 \pm (k_k + k_l + k_r))$

Konsumsi BBM dasar

$$\begin{aligned} \text{Gol I} &= 0.0284V^2 - 3.0644V + 141.68 \\ &= 0.0284(42,47^2) - 3.0644(42,47) + 141.68 \\ &= 62,76 \text{ lt}/1000\text{km} \end{aligned}$$

Konsumsi BBM

$$Y = 62,76 \times (1 + (0,4 + 0,185 + 0,035))$$

$$Y = 101,671 \text{ lt}/1000\text{km}$$

$Y' = \text{Konsumsi BBM} \times \text{Harga BBM}$

$$Y' = 101,671 \times 8500$$

$$= \text{Rp.}864.205,717 / 1000\text{km}$$

2. Konsumsi Oli Mesin

Konsumsi Pelumas = Konsumsi pelumas dasar x faktor koreksi

$$Y = (0,0027 \text{ lt}/\text{km} \times 1000) = 2,7 / 1000\text{km}$$

$Y' = \text{Konsumsi Pelumas} \times \text{Harga Pelumas}$

$$Y' = 2,7 \times 49500 = \text{Rp.}133.650 / 1000\text{km}$$

3. Konsumsi Ban

$$Y = 0,0008848 V - 0,0045333$$

$$Y = 0,0008848 (42,47) - 0,0045333$$

$$Y = 0.033044/1000\text{km}$$

$Y' = Y \times \text{Harga Ban}$

$$Y' = 0.033044 \times 1.100.000$$

$$Y' = \text{Rp.} 36.348,57/1000\text{km}$$

4. Biaya Pemeliharaan (Suku Cadang)

$$Y = 0,0000064 V + 0,0005567$$

$$Y = 0,0000064 (42,47) + 0,0005567$$

$$Y = 0,000829/1000\text{km}$$

$Y' = Y \times \text{Harga Kendaraan}$

$$Y' = 0,000829 \times 190800000$$

$$Y' = \text{Rp.}158.079/1000\text{km}$$

5. Biaya Pemeliharaan (Mekanik)

$$Y = 0,00362 V + 0,36267$$

$$Y = 0,00362 (42,47) + 0,36267$$

$$Y = 0,154298/1000\text{km}$$

$$Y' = Y \times \text{Upah kerja per jam}$$

$$Y' = 0,154298 \times 7500$$

$$Y' = \text{Rp.1.157,236}/1000\text{km}$$

6. Depresiasi

$$Y = 1/(2,5 V + 125)$$

$$Y = 1/(2,5 (42,47) + 125)$$

$$Y = 0.004326/1000\text{km}$$

$$Y' = Y \times \text{setengah harga kendaraan}$$

$$Y' = 0.004326 \times (0,5 \times 190800000)$$

$$Y' = \text{Rp.412.674,381}/1000\text{km}$$

7. Bunga Modal

$$\text{INT} = \text{AINT} / \text{AKM}$$

$$\text{INT} = 0.22\% * \text{Harga kendaraan baru}$$

$$\text{INT} = 0.22\% \times 190800000$$

$$\text{INT} = \text{Rp.419.760}/1000\text{km}$$

8. Asuransi

$$Y = 38/(500 V)$$

$$Y = 38/(500 (42,47))$$

$$Y = 0.001789/1000\text{km}$$

$$Y' = Y \times \text{Harga kendaraan}$$

$$Y' = 0.001789 \times 190800000$$

$$Y' = \text{Rp.341.436,308}/1000\text{km}$$

Biaya Operasional Kendaraan Gol I adalah
 = (Konsumsi Bahan bakar + Konsumsi Oli Mesin +
 Konsumsi Pemakaian Ban + Biaya Pemeliharaan (suku
 cadang) + Biaya Pemeliharaan (montir) + Biaya Penyusutan
 Kendaraan + Biaya Suku Bunga + Biaya Asuransi).
 = (Rp. 864.205,717 + Rp. 133.650 + Rp. 36.348,57+
 Rp. 158.079+ Rp. 1.157,236+ Rp. 412.674,381+
 Rp.419.760+ Rp. 341.436,308)
 = Rp.2.367.311,54

Total BOK/1000km = Rp. 2.367.311,54

Panjang Jalan = 7,5 Km
 Volume Kendaraan (LV) = 7856 Kendaraan/hari
 1Tahun = 365 Hari

BOK/hari = (BOK/1000km x Panjang Jalan x Vol
 Kendaraan)/1000
 = (Rp. 2.367.311,54 x 7,5 km x 7856
 kend/hari)/1000
 = Rp. 139.481.995,9

BOK/Tahun = (BOK/1000km x 365 x Panjang Jalan x Vol
 Kendaraan)/1000
 = (Rp. 2.367.311,54 x 365 x 7,5 km x 7856
 kend/hari)/1000
 = Rp. 50.910.928.512,92

Tabel 4.11 Perhitungan BOK Gol I Jalan Eksisting

Tahun	Jumlah Kendaraan	Panjang Jalan (Km)	Kecepatan Aktual (Km/Jam)	Bahan Bakar	Minyak Pelumas	Ban	Pemeliharaan Suku Cadang	Mekanik	Depresiasi	Bunga Modal	Asuransi	BOK/1000km (Rupiah)	BOK/Hari (Rupiah)	BOK/Tahun (Rupiah)
2014	7856	7.5	42.47	864205.7172	133650	36348.57	158079.3264	1157.236	412674.3809	419760	341436.31	2367311.54	139481995.9	50910928512.92
2015	8508	7.5	40.58	882573.85	133650	34509.07	155771.410	1105.922	421285.0519	419760	357338.59	2405993.90	153517448.28	56033868623.16
2016	9159	7.5	38.74	903140.04	138600	32718.24	153524.549	1055.966	430020.284	419760	374310.79	2453129.86	168511623.19	61506742465.33
2017	9810	7.5	36.70	929037.06	138600	30732.75	151033.464	1000.58	440138.4083	419760	395117.17	2505419.42	184345629.42	67286154738.52
2018	10462	7.5	34.10	966760.91	138600	28202.22	147858.552	929.9903	453745.541	419760	425243.40	2581100.61	202526059.62	73922011762.59
2019	11113	7.5	31.41	1011355.52	138600	25584.09	144573.739	856.9568	468738.4842	419760	461661.89	2671130.69	222642081.93	81264359905.43

Tabel 4.12 Perhitungan BOK Gol I Jalan Rencana

Tahun	Jumlah Kendaraan	Panjang Jalan (Km)	Kecepatan Aktual (Km/Jam)	Bahan Bakar	Minyak Pelumas	Ban	Pemeliharaan Suku Cadang	Mekanik	Depresiasi	Bunga Modal	Asuransi	BOK/1000m (Rupiah)	BOK/Hari (Rupiah)	BOK/Tahun (Rupiah)
2014	7856	7.5	62.74	842871.06	143550	56076.9572	182831.429	1707.566	338477.914	419760	231125.28	2216400.21	130590300.16	47665459557.93
2015	8508	7.5	61.97	837809.62	143550	55327.5316	181891.166	1686.661	340805.573	419760	233997.10	2214827.65	141319846.47	51581743960.55
2016	9159	7.5	61.16	832985.73	143550	54539.1748	180902.059	1664.669	343288.953	419760	237096.14	2213786.73	152070544.95	55505748906.95
2017	9810	7.5	60.27	828277.10	143550	53672.9556	179815.262	1640.506	346059.672	419760	240597.31	2213372.81	162857204.41	59442879610.59
2018	10462	7.5	59.34	824018.76	133650	52767.8052	178679.621	1615.256	349003.110	419760	244368.05	2203862.60	172926079.23	63118018917.81
2019	11113	7.5	58.34	820194.67	133650	51794.5252	177458.501	1588.106	352224.478	419760	248556.74	2205227.02	183808428.37	67090076353.41

Golongan II A

1. Konsumsi Bahan Bakar

$$\text{Konsumsi BBM} = \text{konsumsi BBM dasar} \times (1 \pm (kk + kl + kr))$$

$$\text{KKB dasar golongan I} = 64,73 \text{ lt}/1000\text{km}$$

$$\text{Konsumsi BBM dasar}$$

$$\text{Gol II A} = 2,26533 \times (\text{KBB dasar golongan I})$$

$$= 2,26533 \times 64,73$$

$$= 146,630 \text{ lt}/1000\text{km}$$

$$\text{Konsumsi BBM}$$

$$Y = 139,487 \times (1 + (0,4 + 0,185 + 0,035))$$

$$Y = 237,540 \text{ lt}/1000\text{km}$$

$$Y' = \text{Konsumsi BBM} \times \text{Harga BBM}$$

$$Y' = 237,540 \times 8500$$

$$= \text{Rp. } 1.781.551,14/1000\text{km}$$

2. Konsumsi Oli Mesin

$$\text{Konsumsi Pelumas} = \text{Konsumsi pelumas dasar} \times \text{faktor koreksi}$$

$$Y = (0,0055 \text{ lt}/\text{km} \times 1000) = 5,5 /1000\text{km}$$

$$Y' = \text{Konsumsi Pelumas} \times \text{Harga Pelumas}$$

$$Y' = 5,5 \times 49500 = \text{Rp. } 272.250/1000\text{km}$$

3. Konsumsi Ban

$$Y = 0,0012356 V - 0,0064667$$

$$Y = (0,0012356 (39,77)) - 0,0064667$$

$$Y = 0,030655/1000\text{km}$$

$$Y' = Y \times \text{Harga Ban}$$

$$Y' = 0,030655 \times 1.432.000$$

$$Y' = \text{Rp. } 61.107,90/1000\text{km}$$

4. Biaya Pemeliharaan (Suku Cadang)

$$Y = 0,0000332 V + 0,0020891$$

$$Y = 0,0000332 (39,77) + 0,0020891$$

$$Y = 0,003409/1000\text{km}$$

$$Y' = Y \times \text{Harga Kendaraan}$$

$$Y' = 0,003409 \times 1.212.200.000$$

$$Y' = \text{Rp. } 4.132.952,26/1000\text{km}$$

5. Biaya Pemeliharaan (Mekanik)

$$Y = 0,02311 V + 1,97733$$

$$Y = 0,02311 (39,77) + 1,97733$$

$$Y = 0,921174/1000\text{km}$$

$$Y' = Y \times \text{Upah kerja per jam}$$

$$Y' = 0,921174 \times 7500$$

$$Y' = \text{Rp. } 6.908,8/1000\text{km}$$

6. Depresiasi

$$Y = 1/(9,0 V + 450)$$

$$Y = 1/(9,0 (39,77) + 450)$$

$$Y = 0,001238/1000\text{km}$$

$$Y' = Y \times \text{setengah harga kendaraan}$$

$$Y' = 0,001238 \times (0,5 \times 1.212.200.000)$$

$$Y' = \text{Rp. } 750.188,754/1000\text{km}$$

7. Bunga Modal

$$\text{INT} = \text{AINT} / \text{AKM}$$

$$\text{INT} = 0.22\% * \text{Harga kendaraan baru}$$

$$\text{INT} = 0.22\% \times 1.212.200.000$$

$$\text{INT} = \text{Rp. } 2.666.840/1000\text{km}$$

8. Asuransi

$$Y = 6/(2571,42857 \text{ V})$$

$$Y = 6/(2571,42857 (39,77))$$

$$Y = 0.000587/1000\text{km}$$

$$Y' = Y \times \text{Harga kendaraan}$$

$$Y' = 0.000587 \times 1.212.200.000$$

$$Y' = \text{Rp. } 711.206,102/1000\text{km}$$

Biaya Operasional Kendaraan Gol II A adalah

= (Konsumsi Bahan bakar + Konsumsi Oli Mesin +
Konsumsi Pemakaian Ban + Biaya Pemeliharaan (suku
cadang) + Biaya Pemeliharaan (montir) + Biaya Penyusutan
Kendaraan + Biaya Suku Bunga + Biaya Asuransi).

$$= (\text{Rp. } 1.781.551,14 + \text{Rp. } 272.250 + \text{Rp. } 61.107,90 + \text{Rp. } 4.132.952,26 + \text{Rp. } 6.908,8 + \text{Rp. } 750.188,754 + \text{Rp. } 2.666.840 + \text{Rp. } 711.206,102)$$

$$= \text{Rp. } 10.383.004,961$$

$$\text{Total BOK}/1000\text{km} = \text{Rp. } 10.383.004,961$$

$$\text{Panjang Jalan} = 7,5 \text{ Km}$$

$$\text{Volume Kendaraan (LV)} = 8736 \text{ Kendaraan/hari}$$

$$1\text{Tahun} = 365 \text{ Hari}$$

$$\text{BOK/hari} = (\text{BOK}/1000\text{km} \times \text{Panjang Jalan} \times \text{Vol Kendaraan})/1000$$

$$= (\text{Rp. } 10.383.004,961 \times 7,5 \text{ km} \times 8736 \text{ kend/hari})/1000$$

$$= \text{Rp. } 680.364.570,31$$

$$\text{BOK/Tahun} = (\text{BOK}/1000\text{km} \times 365 \times \text{Panjang Jalan} \times \text{Vol Kendaraan})/1000$$

$$= (\text{Rp. } 10.383.004,961 \times 365 \times 7,5 \text{ km} \times 8736 \text{ kend/hari})/1000$$

$$= \text{Rp. } 248.333.068.164,7$$

Tabel 4.13 Perhitungan BOK Gol II A Jalan Eksisting

Tahun	Jumlah Kendaraan	Panjang Jalan (Km)	Kecepatan Aktual (Km/Jam)	Bahan Bakar	Minyak Pelumas	Ban	Pemeliharaan Suku Cadang	Mekanik	Depresiasi	Bunga Modal	Asuransi	BOK/1000km (Rupiah)	BOK/Hari (Rupiah)	BOK/Tahun (Rupiah)
2014	8737	7.5	39.77	1781551.14	272250	61107.9	4132952.261	6908.804	750188.75	2666840	711206.10	10383004.961	680364570.31	248333068164.70
2015	9407	7.5	38.28	1816318.94	272250	58471.52	4072987.151	6650.549	762850.53	2666840	738888.89	10395257.58	733434799.71	267703701893.75
2016	10078	7.5	36.84	1853218.08	272250	55923.62	4015034.294	6400.961	775500.28	2666840	767770.54	10412937.77	787038472.20	287269042352.67
2017	10748	7.5	35.24	1898019.18	272250	53092.61	3950642.230	6123.641	790056.83	2666840	802629.59	10439654.08	841548345.50	307165146106.76
2018	11419	7.5	33.20	1960945.00	272250	49483.08	3868542.348	5770.058	809428.42	2666840	851947.79	10485206.69	897939994.43	327748097965.86
2019	12089	7.5	31.09	2032874.77	272250	45749.68	3783625.314	5404.343	830490.13	2666840	909767.34	10547001.58	956262355.32	349035759692.71

Tabel 4.14 Perhitungan BOK Gol II A Jalan Rencana

Tahun	Jumlah Kendaraan	Panjang Jalan (Km)	Kecepatan Aktual (Km/Jam)	Bahan Bakar	Minyak Pelumas	Ban	Pemeliharaan Suku Cadang	Mekanik	Depresiasi	Bunga Modal	Asuransi	BOK/1000km (Rupiah)	BOK/Hari (Rupiah)	BOK/Tahun (Rupiah)
2014	8737	7.5	53.41	1624590.841	267300	85242.229	4681894.606	9272.96	651237.25	2666840	529576.23	10515954.12	689076296.73	251512848306.97
2015	9407	7.5	52.76	1625470.550	267300	84092.132	4655735.330	9160.30	655356.60	2666840	536100.58	10500055.49	740828790.22	270402508429.35
2016	10078	7.5	52.07	1627127.132	267300	82871.261	4627966.253	9040.70	659786.86	2666840	543204.66	10484136.86	792419895.49	289233261855.50
2017	10748	7.5	51.31	1629813.178	267300	81526.532	4597380.022	8908.97	664736.40	2666840	551250.57	10467755.67	843813635.59	307991976989.67
2018	11419	7.5	50.52	1633562.412	267300	80128.723	4565586.441	8772.05	669960.65	2666840	559870.68	10452020.95	895098009.20	326710773357.27
2019	12089	7.5	49.67	1638686.054	267300	78624.750	4531378.157	8624.72	675674.17	2666840	569451.71	10436579.57	946250750.38	345381523887.57

Golongan II B

1. Konsumsi Bahan Bakar

$$\text{Konsumsi BBM} = \text{konsumsi BBM dasar} \times (1 \pm (k_k + k_l + k_r))$$

$$\text{KBB dasar golongan I} = 65,288 \text{ lt}/1000\text{km}$$

$$\text{Konsumsi BBM dasar}$$

$$\text{Gol IIB} = 2,90805 \times (\text{KBB dasar golongan I})$$

$$= 2,90805 \times 65,288$$

$$= 189,863 \text{ lt}/1000\text{km}$$

$$\text{Konsumsi BBM}$$

$$Y = 189,863 \times (1 + (0,4 + 0,185 + 0,035))$$

$$Y = 307,577 \text{ lt}/1000\text{km}$$

$$Y' = \text{Konsumsi BBM} \times \text{Harga BBM}$$

$$Y' = 307,577 \times 8500$$

$$= \text{Rp. } 2.306.830 /1000\text{km}$$

2. Konsumsi Oli Mesin

$$\text{Konsumsi Pelumas} = \text{Konsumsi pelumas dasar} \times \text{faktor koreksi}$$

$$Y = (0,0044 \text{ lt}/\text{km} \times 1000) = 4,4 /1000\text{km}$$

$$Y' = \text{Konsumsi Pelumas} \times \text{Harga Pelumas}$$

$$Y' = 4,4 \times 49500 = \text{Rp. } 217.800/1000\text{km}$$

3. Konsumsi Ban

$$Y = 0,0015553 \text{ V} - 0,0059333$$

$$Y = 0,0015553 (39,09) - 0,0059333$$

$$Y = 0,054863/1000\text{km}$$

$$Y' = Y \times \text{Harga Ban}$$

$$Y' = 0,054863 \times 1.446.000$$

$$Y' = \text{Rp. } 79.332,44/1000\text{km}$$

4. Biaya Pemeliharaan (Suku Cadang)
 $Y = 0,0000191 V + 0,0015400$
 $Y = 0,0000191 (39,09) + 0,0015400$
 $Y = 0.002287/1000\text{km}$

$$Y' = Y \times \text{Harga Kendaraan}$$

$$Y' = 0.002287 \times 11187000000$$

$$Y' = \text{Rp. } 2.558.040,675 /1000\text{km}$$

5. Biaya Pemeliharaan (Mekanik)
 $Y = 0,01511 V + 1,21200$
 $Y = 0,01511 (39,09) + 1,21200$
 $Y = 0.592190/1000\text{km}$

$$Y' = Y \times \text{Upah kerja per jam}$$

$$Y' = 0.592190 \times 7500$$

$$Y' = \text{Rp. } 4.441,424/1000\text{km}$$

6. Depresiasi
 $Y = 1/(6,0 V + 300)$
 $Y = 1/(6,0 (39,09) + 300)$
 $Y = 0.001871/1000\text{km}$

$$Y' = Y \times \text{setengah harga kendaraan}$$

$$Y' = 0.001871 \times (0,5 \times 11187000000)$$

$$Y' = \text{Rp. } 1.046.413,739/1000\text{km}$$

7. Bunga Modal
 $\text{INT} = \text{AINT} / \text{AKM}$

$$\text{INT} = 0.22\% * \text{Harga kendaraan baru}$$

$$\text{INT} = 0.22\% \times 11187000000$$

$$\text{INT} = \text{Rp. } 2.461.140/1000\text{km}$$

8. Asuransi

$$Y = 61 / (1714,28571 \text{ V})$$

$$Y = 61 / (1714,28571 (39,09))$$

$$Y = 0.000910 / 1000\text{km}$$

$$Y' = Y \times \text{Harga kendaraan}$$

$$Y' = 0.000910 \times 11187000000$$

$$Y' = \text{Rp. } 1.018.344,208 / 1000\text{km}$$

Biaya Operasional Kendaraan Gol II B adalah

= (Konsumsi Bahan bakar + Konsumsi Oli Mesin +
Konsumsi Pemakaian Ban + Biaya Pemeliharaan (suku
cadang) + Biaya Pemeliharaan (montir) + Biaya Penyusutan
Kendaraan + Biaya Suku Bunga + Biaya Asuransi).

$$= (\text{Rp. } 2.306.830 + \text{Rp. } 217.800 + \text{Rp. } 79.332,44 + \text{Rp. } 2.558.040,675 + \text{Rp. } 4.441,424 + \text{Rp. } 1.046.413,739 + \text{Rp. } 2.461.140 + \text{Rp. } 1.018.344,208)$$

$$= \text{Rp. } 9.692.342,1$$

$$\text{Total BOK}/1000\text{km} = \text{Rp. } 9.692.342,1$$

$$\text{Panjang Jalan} = 7,5 \text{ Km}$$

$$\text{Volume Kendaraan (LV)} = 864 \text{ Kendaraan/hari}$$

$$1\text{Tahun} = 365 \text{ Hari}$$

$$\text{BOK/hari} = (\text{BOK}/1000\text{km} \times \text{Panjang Jalan} \times \text{Vol}$$

$$\text{Kendaraan}) / 1000$$

$$= (\text{Rp. } 9.692.342,1 \times 7,5 \text{ km} \times 864$$

$$\text{kend/hari}) / 1000$$

$$= \text{Rp. } 62.806.376,79$$

$$\text{BOK/Tahun} = (\text{BOK}/1000\text{km} \times 365 \times \text{Panjang Jalan} \times \text{Vol}$$

$$\text{Kendaraan}) / 1000$$

$$= (\text{Rp. } 9.692.342,1 \times 365 \times 7,5 \text{ km} \times 864$$

$$\text{kend/hari}) / 1000$$

$$= \text{Rp. } 22.924.327.529,38$$

Tabel 4.15 Perhitungan BOK Gol II B Jalan Eksisting

Tahun	Jumlah Kendaraan	Panjang Jalan (Km)	Kecepatan Aktual (Km/Jam)	Bahan Bakar	Minyak Pelumas	Ban	Pemeliharaan Suku Cadang	Mekanik	Depresiasi	Bunga Modal	Asuransi	BOK/1000km (Rupiah)	BOK/Hari (Rupiah)	BOK/Tahun (Rupiah)
2014	864	7.5	39.09	2306829.608	217800	79332.443	2558040.675	4441.424	1046413.739	2461140	1018344.2	9692342.10	62806376.79	22924327529.38
2015	943	7.5	37.71	2349897.700	217800	76228.873	2528553.981	4285.036	1062877.665	2461140	1055610.6	9756393.84	68994778.12	25183094014.28
2016	1022	7.5	36.37	2395374.822	217800	73215.262	2499921.973	4133.180	1079367.836	2461140	1094503	9825456.10	75297382.82	27483544729.95
2017	1101	7.5	34.88	2450173.922	217800	69864.306	2468084.890	3964.326	1098315.269	2461140	1141257.9	9910600.60	81814485.39	29862287240.60
2018	1180	7.5	32.97	2526936.163	217800	65568.785	2427273.595	3747.875	1123598.891	2461140	1207372.6	10033437.92	88765825.31	32399526238.37
2019	1258	7.5	31.01	2613319.269	217800	61160.816	2385393.942	3525.758	1150783.854	2461140	1283685.1	10176808.74	96056353.53	35060569038.33

Tabel 4.16 Perhitungan BOK Gol II B Jalan Rencana

Tahun	Jumlah Kendaraan	Panjang Jalan (Km)	Kecepatan Aktual (Km/Jam)	Bahan Bakar	Minyak Pelumas	Ban	Pemeliharaan Suku Cadang	Mekanik	Depresiasi	Bunga Modal	Asuransi	BOK/1000km (Rupiah)	BOK/Hari (Rupiah)	BOK/Tahun (Rupiah)
2014	864	7.5	50.87	2094750.293	212850	105825.237	2809745.938	5776.393	924209.378	2461140	782525.557	9396822.80	60891411.72	22225365277.76
2015	943	7.5	50.24	2099043.649	212850	104408.390	2796284.621	5704.998	930017.957	2461140	792338.278	9401787.89	66487093.53	24267789138.16
2016	1022	7.5	49.59	2104308.174	212850	102946.563	2782395.960	5631.337	936087.961	2461140	802723.837	9408083.83	72098850.45	26316080412.97
2017	1101	7.5	48.57	2114278.708	212850	100652.620	2760601.447	5515.745	945774.576	2461140	819581.534	9420394.63	77767712.77	28385215161.04
2018	1180	7.5	48.11	2119458.373	212850	99618.097	2750772.549	5463.616	950208.949	2461140	827417.899	9426929.48	83400045.12	30441016470.26
2019	1258	7.5	47.30	2129611.338	212850	97796.436	2733465.141	5371.823	958119.219	2461140	841587.211	9439941.17	89101244.70	32521954316.16

Sepeda Motor (MC)

Pada perhitungan metode Jasa Marga tidak dibahas secara khusus untuk jenis kendaraan sepeda motor. Oleh sebab itu untuk pengaruh jenis kendaraan MC terhadap LV dihitung sebagai berikut:

MC = 7820 kend/hari, LV = 7856 kend/hari

Perbandingan MC dengan LV adalah $100/7856 \times 7820 = 99,54$

Faktor penyesuaian $0.18 \times 99,54/100 = 0.179$

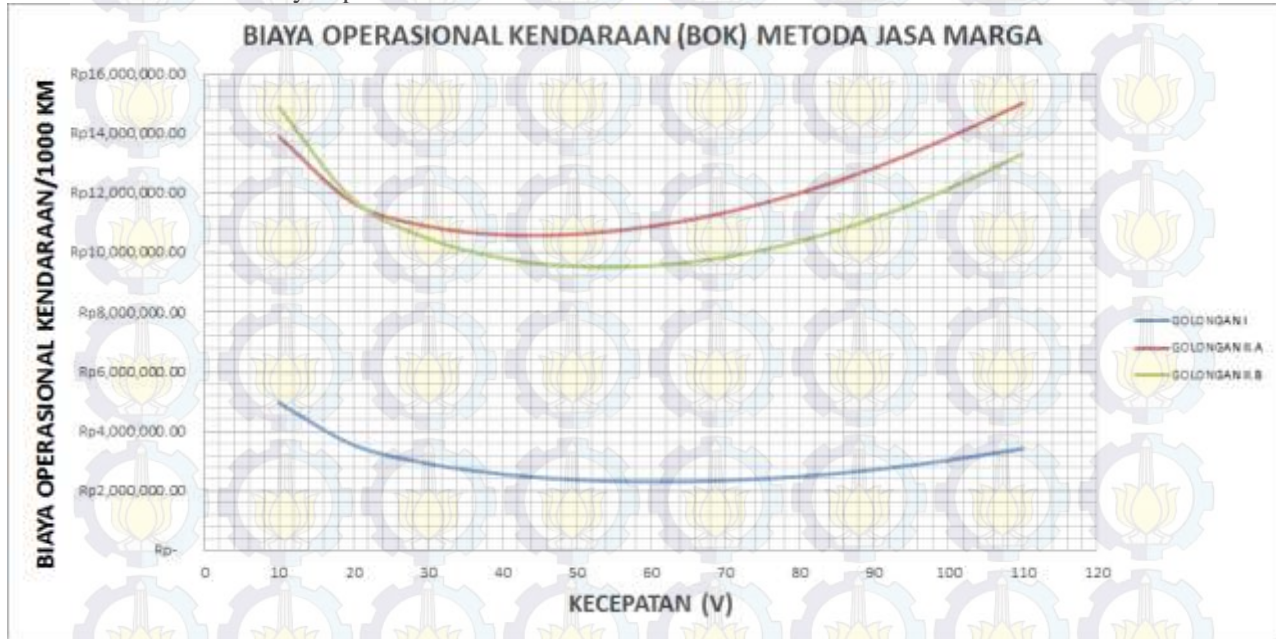
Akibat adanya sepeda motor biaya operasi kendaraan auto/tahun akan dikalikan dengan $1 + (0.18 \times 99,682)/100 = 1,179$

(Sumber : Kartika, Gde, A Agung 2006. Diktat ekonomi jalan raya jurusan teknik ITS Surabaya)

Contoh perhitungan = $1.179 \times \text{Rp. } 50.910.928.512,923$
= Rp. 60.023.984.716,736

Dengan memasukan besarnya kecepatan kendaraan dan besarnya volume lalu lintas yang didapat dari output KAJI, didapatkan BOK eksisting dan BOK rencana dengan cara menarik kecepatan pada Gambar 4.3 grafik BOK, kemudian didapatkan besarnya BOK untuk masing – masing jenis kendaraan. Besarnya BOK akan ditabelkan pada Tabel 4.16, Tabel 4.17 dan Tabel 4.18 dibawah ini.

Gambar 4.10 Grafik biaya operasional kendaraan



Tabel 4.17 Biaya Operasional Kendaraan/tahun Pada Jalan Eksisting

Tahun	Gol I	Gol II A	Gol II B	Sepeda Motor	TOTAL
2014	Rp 50,910,928,512.923	Rp 248,333,068,164.698	Rp 22,924,327,529.376	Rp 60,023,984,716.74	Rp 382,192,308,923.732
2015	Rp 56,033,868,623.156	Rp 267,703,701,893.753	Rp 25,183,094,014.282	Rp 66,063,931,106.70	Rp 414,984,595,637.893
2016	Rp 61,506,742,465.332	Rp 287,269,042,352.674	Rp 27,483,544,729.946	Rp 72,516,449,366.63	Rp 448,775,778,914.578
2017	Rp 67,286,154,738.523	Rp 307,165,146,106.755	Rp 29,862,287,240.600	Rp 79,330,376,436.72	Rp 483,643,964,522.597
2018	Rp 73,922,011,762.590	Rp 327,748,097,965.855	Rp 32,399,526,238.367	Rp 87,154,051,868.09	Rp 521,223,687,834.906
2019	Rp 81,264,359,905.434	Rp 349,035,759,692.710	Rp 35,060,569,038.335	Rp 95,810,680,328.51	Rp 561,171,368,964.985

Tabel 4.18 Biaya Operasional Kendaraan/tahun Pada Jalan Rencana

Tahun	Gol I	Gol II A	Gol II B	Sepeda Motor	TOTAL
2014	Rp 47,665,459,557.929	Rp 251,512,848,306.965	Rp 22,225,365,277.765	Rp 56,197,576,818.80	Rp 377,601,249,961.456
2015	Rp 51,581,743,960.546	Rp 270,402,508,429.353	Rp 24,267,789,138.160	Rp 60,814,876,129.48	Rp 407,066,917,657.543
2016	Rp 55,505,748,906.950	Rp 289,233,261,855.498	Rp 26,316,080,412.972	Rp 65,441,277,961.29	Rp 436,496,369,136.713
2017	Rp 59,442,879,610.590	Rp 307,991,976,989.673	Rp 28,385,215,161.041	Rp 70,083,155,060.89	Rp 465,903,226,822.191
2018	Rp 63,118,018,917.809	Rp 326,710,773,357.266	Rp 30,441,016,470.264	Rp 74,416,144,304.10	Rp 494,685,953,049.436
2019	Rp 67,090,076,353.407	Rp 345,381,523,887.565	Rp 32,521,954,316.159	Rp 79,099,200,020.67	Rp 524,092,754,577.797

Tabel 4.19 Saving biaya operasional kendaraan

Tahun	Saving
2014	Rp 4,591,058,962.276
2015	Rp 7,917,677,980.350
2016	Rp 12,279,409,777.865
2017	Rp 17,740,737,700.406
2018	Rp 26,537,734,785.470
2019	Rp 37,078,614,387.188

4.5.2 Analisa Biaya Nilai Waktu

Nilai waktu adalah sejumlah uang yang dikeluarkan seseorang untuk menghemat satu unit perjalanan. Manfaat dari nilai waktu pada dasarnya yaitu penghematan waktu perjalanan yang dinilai secara ekonomis. Nilai waktu biasanya sebanding dengan pendapatan perkapita suatu daerah.

Nilai waktu besarnya berbeda - beda menurut jenis kendaraan dan lokasi studi. Pada tugas akhir ini metode perhitungan nilai waktu menggunakan PT. Jasa Marga (1990-1996). Rumus yang digunakan yaitu sebagai berikut:

$$\text{Nilai Waktu} : \text{Max} \{ (K * \text{Nilai Waktu Dasar}) ; \text{Nilai Waktu Minimum} \}$$

Nilai K adalah nilai faktor koreksi dengan asumsi bahwa nilai waktu dasar tersebut hanya berlaku untuk daerah DKI Jakarta dan sekitarnya. Untuk faktor koreksi pada daerah selain Jakarta yaitu menggunakan nilai K pada tabel 4.10.

Tabel 4.20 Nilai waktu dasar dari berbagai studi.

Referensi	Nilai Waktu (Rp/Jam/kend)		
	Gol I	Gol IIa	Gol IIb
PT. Jasa Marga (1990-1996), Formula Herbert Mohring	12.287.00	18.534.00	13.768.00
Padalarang-Cileunyi (1996)	3385 - 5425	3827 - 38344	5.716.00
Semarang -1996	3411 - 6221	14.541.00	1.506.00
IHCM (1995)	3281,25	18.212.00	4971,20
PCI (1979)	1.341.00	3.827.00	3.152.00
JIUTR northern extension (PCI 1989)	7.067.00	14.670.00	3.659.00
Surabaya-Mojokerto (JICA 1991)	8.880.00	7.960.00	7.980.00

Sumber : Tamin (2000)

Tabel 4.21 Besarnya Nilai waktu Minimum

No	Kab/Kota	Jasa Marga			JIUTR		
		Gol I	Gol IIa	Gol IIb	Gol I	Gol IIa	Gol IIb
1	DKI	8200	12369	9188	8200	17022	4246
2	Selain DKI	6000	9051	6723	6000	12455	3170

Sumber : Tamin (2000)

Tabel 4.22 Nilai koreksi (K)

No	Lokasi	Nilai koreksi
1	DKI Jakarta	1,00
2	Jawa Barat	0,23
3	Kodya	0,39
4	Jawa Tengah	0,20
5	Kodya	0,52
6	Jawa Timur	0,25
7	Kodya	0,74
8	Sumatera	0,29
9	Kodya Medan	0,46

Tabel 4.23 Nilai waktu kendaraan (Rp/Jam/Kend)

no	Jenis Kendaraan	K	Nilai Waktu Dasar	Nilai waktu	Nilai Waktu Minimum	Nilai Waktu yang dipakai
1	Gol I	0.25	Rp 12,287.00	Rp 3,071.75	Rp 6,000.00	Rp 6,000.00
2	Gol IIa	0.25	Rp 18,534.00	Rp 4,633.50	Rp 9,051.00	Rp 9,051.00
3	Gol IIb	0.25	Rp 13,768.00	Rp 3,442.00	Rp 6,723.00	Rp 6,723.00

Harga nilai waktu yang dipakai diatas merupakan harga nilai waktu pada saat tahun 1996. Harga nilai tersebut perlu dikonversikan ke tahun sekarang (2014), dengan mengalikan faktor bunga selama 18 tahun. Nilai suku bunga yang dipakai yaitu pada tabel 4.12. (*sumber* , www.bi.go.id)

Tabel 4.24 Nilai Suku Bunga per Tahun

Tahun	BI rate I (%)
1996	
1997	21.89
1998	21.89
1999	21.89
2000	10.99
2001	10.99
2002	10.99
2003	10.99
2004	10.99
2005	12.75
2006	9.75
2007	8
2008	9.25
2009	6.5
2010	6.5
2011	6
2012	5.75
2013	7.5
2014	7.75

Tabel 4.25 Nilai waktu kendaraan tahun 2014-2034
(Rp/Jam/Kend)

Tahun	Gol I	Gol Iia	Gol Iib
1996	6000	9051	6723
2014	29261.186	59498.545	44194.975
2015	31528.927	64109.683	47620.086
2016	33972.419	69078.183	51310.642
2017	36605.282	74431.742	55287.217
2018	39442.191	80200.202	59571.977
2019	42498.961	86415.718	64188.805
2020	45792.630	93112.936	69163.437
2021	49341.559	100329.189	74523.603
2022	53165.530	108104.701	80299.183
2023	57285.859	116482.815	86522.369
2024	61725.513	125510.233	93227.853
2025	66509.240	135237.276	100453.012
2026	71663.706	145718.165	108238.120
2027	77217.643	157011.323	116626.574
2028	83202.011	169179.700	125665.134
2029	89650.167	182291.127	135404.182
2030	96598.054	196418.690	145898.006
2031	104084.404	211641.138	157205.101
2032	112150.945	228043.326	169388.497
2033	120842.643	245716.684	182516.105
2034	130207.948	264759.727	196661.103

Nilai waktu = Selisih waktu antara rencana dengan waktu Eksisting x Jumlah Kendaraan x Nilai waktu.

Nilai selisih waktu rencana dengan waktu eksisting didapatkan dari nilai dari bantuan program KAJI yang akan ditabelkan pada tabel 4.13.

Tabel 4.26 Selisih waktu rencana dengan waktu eksisting kendaraan

Tahun	Selisih Waktu GOL I (jam)	Selisih Waktu GOL II A (jam)	Selisih Waktu GOL II B (jam)
2014	0.057	0.048	0.044
2015	0.064	0.054	0.050
2016	0.071	0.060	0.055
2017	0.080	0.067	0.061
2018	0.094	0.077	0.072
2019	0.110	0.090	0.083

Contoh hasil perhitungan nilai waktu pada masing-masing kendaraan

$$\begin{aligned}
 &\text{Gol I} \\
 &\text{Nilai Waktu/Tahun} = \text{Selisih waktu} \times 365 \times \text{Nilai Waktu} \times \\
 &\quad \text{Jumlah Kendaraan/hari} \\
 &= 0,057 \times 365 \times \text{Rp.}29.261,86 \times 7856 \\
 &= \text{Rp.} 4.786.389.879,18
 \end{aligned}$$

$$\begin{aligned}
 &\text{Gol II A} \\
 &\text{Nilai Waktu/Tahun} = \text{Selisih waktu} \times 365 \times \text{Nilai Waktu} \times \\
 &\quad \text{Jumlah Kendaraan/hari} \\
 &= 0,048 \times 365 \times \text{Rp.}59.498,545 \times 8737 \\
 &= \text{Rp.} 9.138.060.778,364
 \end{aligned}$$

$$\begin{aligned}
 &\text{Gol II B} \\
 &\text{Nilai Waktu/Tahun} = \text{Selisih waktu} \times 365 \times \text{Nilai Waktu} \times \\
 &\quad \text{Jumlah Kendaraan/hari} \\
 &= 0,044 \times 365 \times \text{Rp.}44.194,975 \times 864 \\
 &= \text{Rp.} 619.239.494,158
 \end{aligned}$$

MC = 7820 kend/hari, LV = 7856 kend/hari

Perbandingan MC dengan LV adalah $100/7856 \times 7820 = 99,54$

Faktor penyesuaian $0.18 \times 99,54 / 100 = 0.179$

Akibat adanya sepeda motor biaya operasi kendaraan auto/tahun akan dikalikan dengan $1 + (0.18 \times 99,682) / 100 = 1,179$

Contoh perhitungan = $1,179 \times \text{Rp. } 4.786.389.879,18$
= Rp 5.643.153.667,554

Tabel 4.27 Nilai waktu per tahun tiap jenis kendaraan

Tahun	Gol I	Gol IIa	Gol IIb	MC	Total Saving per Tahun
2014	Rp 4,786,389,879.18	Rp 9,138,060,778.36	Rp 619,239,494.16	Rp 5,643,153,667.55	Rp 14,543,690,151.70
2015	Rp 6,246,255,768.33	Rp 11,836,807,767.99	Rp 812,933,170.74	Rp 7,364,335,550.86	Rp 18,895,996,707.06
2016	Rp 8,061,270,306.39	Rp 15,130,356,592.71	Rp 1,052,014,011.60	Rp 9,504,237,691.24	Rp 24,243,640,910.70
2017	Rp 10,477,300,042.54	Rp 19,463,546,333.86	Rp 1,346,191,034.00	Rp 12,352,736,750.15	Rp 31,287,037,410.40
2018	Rp 14,092,675,579.18	Rp 25,887,201,825.56	Rp 1,836,125,967.45	Rp 16,615,264,507.85	Rp 41,816,003,372.19
2019	Rp 19,002,882,779.78	Rp 34,408,400,362.25	Rp 2,455,979,931.37	Rp 22,404,398,797.36	Rp 55,867,263,073.40

4.6 Penghematan

Penghematan yang terjadi dalam proyek pelebaran jalan Bulu – Tuban adalah sebagai berikut :

Tabel 4.28 Penghematan Total

Tahun	Penghematan BOK	Penghematan Nilai Waktu	Penghematan Total
2014	Rp 4,591,058,962.276	Rp 14,543,690,151.705	Rp 19,134,749,113.981
2015	Rp 7,917,677,980.350	Rp 18,895,996,707.057	Rp 26,813,674,687.407
2016	Rp 12,279,409,777.865	Rp 24,243,640,910.704	Rp 36,523,050,688.570
2017	Rp 17,740,737,700.406	Rp 31,287,037,410.400	Rp 49,027,775,110.806
2018	Rp 26,537,734,785.470	Rp 41,816,003,372.188	Rp 68,353,738,157.659
2019	Rp 37,078,614,387.188	Rp 55,867,263,073.398	Rp 92,945,877,460.586
		Total	Rp 292,798,865,219.009

Penghematan total yang diperoleh dari proyek pelebaran jalan Bulu – Tuban adalah sebesar Rp.292.789.865.219,009

4.7 Rencana Anggaran Biaya (RAB)

Biaya konstruksi pelebaran jalan Bulu Tuban merupakan biaya investasi (cost). Hasil rekapitulasi RAB dapat dilihat pada tabel 4.28 dan 4.29.

Perhitungan volume pekerjaan

1. Galian tanah

Galian pada ruas kiri jalan

$$\begin{aligned} \text{Galian} &= \text{tinggi galian} \times \text{lebar galian} \times \text{panjang jalan} \\ &= 1,3 \times 3,7 \times 7500 = 36.075 \text{ m}^3 \end{aligned}$$

Galian pada ruas kanan jalan

$$\begin{aligned} \text{Galian} &= \text{tinggi galian} \times \text{lebar galian} \times \text{panjang jalan} \\ &= 1,3 \times 3,8 \times 7500 = 37.050 \text{ m}^3 \end{aligned}$$

2. Pekerjaan perkerasan dan bahu jalan

Lapis pondasi agregat kelas B (t =50cm)

Pondasi pada ruas kiri jalan

$$\begin{aligned} \text{Volume} &= \text{tebal pondasi} \times \text{lebar} \times \text{panjang jalan} \\ &= 0,5 \times 3,7 \times 7500 = 13.875 \text{ m}^3 \end{aligned}$$

Pondasi pada ruas kanan jalan

$$\begin{aligned} \text{Volume} &= \text{tebal pondasi} \times \text{lebar} \times \text{panjang jalan} \\ &= 0,5 \times 3,8 \times 7500 = 14.250,5 \text{ m}^3 \end{aligned}$$

Lapis pondasi agregat kelas S (t =80cm)

Pondasi pada ruas kiri jalan

$$\begin{aligned} \text{Volume} &= \text{tebal pondasi} \times \text{lebar} \times \text{panjang jalan} \\ &= 0,8 \times 3,7 \times 7500 = 22.200 \text{ m}^3 \end{aligned}$$

Pondasi pada ruas kanan jalan

$$\begin{aligned} \text{Volume} &= \text{tebal pondasi} \times \text{lebar} \times \text{panjang jalan} \\ &= 0,8 \times 3,8 \times 7500 \times 1,67 = 22.800 \text{ m}^3 \end{aligned}$$

3. Pekerjaan perkerasan berbutir

Lapis pondasi atas CTB (Cement Treated Base) (t=25cm)

Pondasi pada ruas kiri jalan

$$\begin{aligned}\text{Volume} &= \text{tebal pondasi} \times \text{lebar} \times \text{panjang jalan} \\ &= 0,25 \times 3,7 \times 7500 = 6937,5 \text{ m}^3\end{aligned}$$

Pondasi pada ruas kanan jalan

$$\begin{aligned}\text{Volume} &= \text{tebal pondasi} \times \text{lebar} \times \text{panjang jalan} \\ &= 0,25 \times 3,8 \times 7500 = 7125 \text{ m}^3\end{aligned}$$

4. Pekerjaan perkerasan aspal

Lapis perekat aspal (liter)

Prime coat (1,3lt/m²)

$$\text{Volume} = 2,26 \times 7500 \times 1,3 = 22.035 \text{ lt}$$

Tack coat (0,5lt/m²)

$$\text{Volume} = 2,26 \times 7500 \times 0,5 = 8.475 \text{ lt}$$

Aspal modifikasi crumb rubber (SMA Mod. CR) (t=5cm)

$$\begin{aligned}\text{Volume} &= \text{tebal pondasi} \times \text{lebar} \times \text{panjang jalan} \\ &= 0,05 \times 2,26 \times 7500 \times 2 = 1695 \text{ m}^3\end{aligned}$$

Laston crumb rubber (AC-BC Mod.CR) (t=6cm)

$$\begin{aligned}\text{Volume} &= \text{tebal pondasi} \times \text{lebar} \times \text{panjang jalan} \\ &= 0,06 \times 2,26 \times 7500 \times 2 = 2034 \text{ m}^3\end{aligned}$$

Perhitungan harga satuan

Tabel 4.29 Harga sewa alat per jam

No.	Uraian	Kapasitas	Biaya sewa alat (per jam)
1	DUMP TRUCK	12 Ton	Rp 636,679.685
2	EXCAVATOR 80-140 HP	0.93 M3	Rp 627,539.776
3	DUMP TRUCK 3 - 4 m ³	6 Ton	Rp 292,647.820
4	MOTOR GRADER >100 HP	800	Rp 800,538.908
5	TANDEM ROLLER 6-8 T.	8.1 Ton	Rp 569,325.862
6	WATER TANKER 3000-4500 L.	5000 Liter	Rp 254,606.403
7	WHEEL LOADER 1.0-1.6 M ³	1.5 M ³	Rp 598,547.301
8	VIBRATORY ROLLER 5-8 T.	7.05 Ton	Rp 569,325.862
9	CTB PLANT		Rp 332,057.000
10	ASPHALT DISTRIBUTOR	4000 Liter	Rp 373,066.072
11	COMPRESSOR 4000-6500 L/M	5000 L/m	Rp 171,116.579
12	ASPHALT MIXING PLANT	50 T/Jam	Rp 6,396,132.331
13	GENERATOR SET	395 KVA	Rp 485,626.547
14	ASPHALT FINISHER	10 Ton	Rp 1,560,011.961
15	TIRE ROLLER 8-10 T.	9 Ton	Rp 559,704.294

Tabel 4.30 Harga bahan

No.	Uraian	Satuan	Harga Satuan (Rp.)
1	Agregat Base Kelas B	m ³	189304.5
2	Agregat Base Kelas B	m ³	96558
3	Bahan Agr.Base Kelas A	m ³	198198
4	Semen / PC (kg)	Kg	1504.751
5	Aspal Modifikasi (BNA)	kg	10390.341
6	Agregat Pecah Mesin 0 - 5 mm / Abu Batu	m ³	218526
7	Agregat Pecah Mesin 5 - 10 mm	m ³	213444
8	Agregat Pecah Mesin 10 - 20 mm	m ³	213444
9	Aspal Asbuton crumb rubber	Kg	10530

Tabel 4.31 Harga Upah

No.	Uraian	Satuan	Harga Satuan (Rp.)
1	Pekerja	Jam	6931.767
2	Tukang	Jam	9299.3865
3	M a n d o r	Jam	10655.8245
4	Operator	Jam	12286.8285
5	Pembantu Operator	Jam	10336.1805
6	Sopir / Driver	Jam	12000.993
7	Pembantu Sopir / Driver	Jam	9951.993
8	Mekanik	Jam	13171.9965
9	Pembantu Mekanik	Jam	11122.9965
10	Kepala Tukang	Jam	9953.0175

Tabel 4.32 Harga Satuan Pekerjaan Galian

Galian Biasa					
Kebutuhan		Satuan	Indeks	Harga Satuan	Jml. Harga
Alat	Excavator	Jam	0.01	Rp 627,539.78	Rp 6,829.04
	Dump Truck	Jam	0.18	Rp 636,679.68	Rp 114,716.57
Upah	Pekerja	Jam	0.02	Rp 6,931.77	Rp 138.64
	Mandor	Jam	0.01	Rp 10,655.82	Rp 106.56
				Jumlah	Rp 121,790.80

Tabel 4.33 Harga Satuan Pekerjaan Pondasi Agregat Klas B

Lapis Pondasi Agregat Klas B					
Kebutuhan		Satuan	Indeks	Harga Satuan	Jml. Harga
Alat	Wheel Loader	Jam	0.0071	Rp 598,547.30	Rp 4,242.01
	Dump Truck	Jam	0.429	Rp 636,679.68	Rp 273,439.14
	Motor Grader	Jam	0.004	Rp 800,538.91	Rp 3,409.87
	Tandem Roller	Jam	0.005	Rp 569,325.86	Rp 3,048.60
	Water Tanker	Jam	0.014	Rp 254,606.40	Rp 3,578.80
Bahan	Agregat klas B	m ³	1.20	Rp 189,304.50	Rp 227,165.40
Upah	Pekerja	Jam	0.05	Rp 6,931.77	Rp 343.89
	Mandor	Jam	0.01	Rp 10,655.82	Rp 75.52
				Jumlah	Rp 515,303.22

Tabel 4.34 Harga Satuan Pekerjaan Pondasi Agregat Klas S

Lapis Pondasi Agregat Klas S					
Kebutuhan		Satuan	Indeks	Harga Satuan	Jml. Harga
Alat	Wheel Loader	Jam	0.007	Rp 598,547.30	Rp 4,242.01
	Dump Truck	Jam	0.430	Rp 292,647.82	Rp 125,757.06
	Motor Grader	Jam	0.003	Rp 800,538.91	Rp 2,557.40
	Tandem Roller	Jam	0.004	Rp 569,325.86	Rp 2,286.45
	Water Tanker	Jam	0.014	Rp 254,606.40	Rp 3,578.80
Bahan	Agregat klas S	m ³	1.20	Rp 96,558.00	Rp 115,869.60
Upah	Pekerja	Jam	0.05	Rp 6,931.77	Rp 343.89
	Mandor	Jam	0.01	Rp 10,655.82	Rp 75.52
				Jumlah	Rp 254,710.72

Tabel 4.35 Harga Satuan Pekerjaan Pondasi Atas CTB

Lapis Pondasi atas bersemen (Cement Treated Base / C T B)					
Kebutuhan		Satuan	Indeks	Harga Satuan	Jml. Harga
Alat	Dump Truck	Jam	0.275	Rp 292,647.82	Rp 80,470.80
	Motor Grader	Jam	0.003	Rp 800,538.91	Rp 2,572.01
	Vibrator Roller	Jam	0.002	Rp 569,325.86	Rp 894.70
	Water Tanker	Jam	0.004	Rp 254,606.40	Rp 964.13
	CTB Plant	Jam	0.117	Rp 332,057.00	Rp 38,895.57
Bahan	Agregat Kelas A	m ³	1.05	Rp 198,198.00	Rp 208,107.90
	Semen (PC)	Kg	122.64	Rp 1,504.75	Rp 184,542.66
Upah	Pekerja	Jam	0.05	Rp 6,931.77	Rp 343.89
	Mandor	Jam	0.01	Rp 10,655.82	Rp 75.52
				Jumlah	Rp 516,867.18

Tabel 4.36 Harga Satuan Pekerjaan Lapis Perekat Prime coat

Lapis Perekat - prime coat					
Kebutuhan		Satuan	Indeks	Harga Satuan	Jml. Harga
Alat	Asp. Distributor	Jam	0.0002	Rp 373,066.07	Rp 74.91
	Compressor	Jam	0.0002	Rp 171,116.58	Rp 34.36
Bahan	Aspal	Kg	1.061	Rp 10,390.34	Rp 11,023.11
Upah	Pekerja	Jam	0.0008	Rp 6,931.77	Rp 5.57
	Mandor	Jam	0.0002	Rp 10,655.82	Rp 2.14
				Jumlah	Rp 11,140.09

Tabel 4.37 Harga Satuan Pekerjaan Lapis Perekat Prime coat

Lapis Perekat - tack coat					
Kebutuhan		Satuan	Indeks	Harga Satuan	Jml. Harga
Alat	Asp. Distributor	Jam	0.0002	Rp 373,066.07	Rp 74.91
	Compressor	Jam	0.0002	Rp 171,116.58	Rp 34.36
Bahan	Aspal	Kg	1.061	Rp 10,390.34	Rp 11,023.11
Upah	Pekerja	Jam	0.0008	Rp 6,931.77	Rp 5.57
	Mandor	Jam	0.0002	Rp 10,655.82	Rp 2.14
				Jumlah	Rp 11,140.09

Tabel 4.38 Harga Satuan Pekerjaan Split Matrik Aspal Modifikasi Crumb Rubber (SMA Mod. CR)

Split Matrik Aspal Modifikasi Crumb Rubber (SMA Mod. CR)					
Kebutuhan		Satuan	Indeks	Harga Satuan	Jml. Harga
Alat	AMP	Jam	0.055	Rp6,396,132.33	Rp 352,943.21
	Genset	Jam	0.055	Rp 485,626.55	Rp 26,797.22
	Dump Truck	Jam	0.584	Rp 292,647.82	Rp 171,017.29
	Asp. Finisher	Jam	0.025	Rp1,560,011.96	Rp 39,778.47
	Tandem Roller	Jam	0.025	Rp 569,325.86	Rp 14,290.31
	P. Tyre Roller	Jam	0.011	Rp 559,704.29	Rp 6,027.64
Bahan	Agr 5-10 & 10-20	m ³	0.71	Rp 213,444.00	Rp 151,799.27
	Agr 0-5	m ³	0.76	Rp 218,526.00	Rp 167,004.74
	Asphalt Asbuton Crumb Rubber	Kg	185.49	Rp 10,530.00	Rp 1,953,209.70
Upah	Pekerja	Jam	0.55	Rp 6,931.77	Rp 3,825.00
	Mandor	Jam	0.06	Rp 10,655.82	Rp 588.00
				Jumlah	Rp 2,887,280.84

Tabel 4.39 Harga Satuan Pekerjaan Laston (AC-BC Mod. CR)

Laston Lapis Antara Modifikasi Crumb Rubber (AC-BC Mod.CR)					
Kebutuhan		Satuan	Indeks	Harga Satuan	Jml. Harga
Alat	AMP	Jam	0.056	Rp6,396,132.33	Rp 357,566.92
	Genset	Jam	0.056	Rp 485,626.55	Rp 27,148.28
	Dump Truck	Jam	0.592	Rp 292,647.82	Rp 173,257.70
	Asphalt Finisher	Jam	0.021	Rp1,560,011.96	Rp 33,148.72
	Tandem Roller	Jam	0.021	Rp 569,325.86	Rp 11,908.59
	P. Tyre Roller	Jam	0.009	Rp 559,704.29	Rp 5,023.04
Bahan	Agr 5-10 & 10-20	m ³	0.88	Rp 213,444.00	Rp 187,004.85
	Agr 0-5	m ³	0.66	Rp 218,526.00	Rp 144,780.02
	Asphalt Asbuton Crumb Rubber	Kg	157.71	Rp 10,530.00	Rp 1,660,724.21
Upah	Pekerja	Jam	0.559	Rp 6,931.77	Rp 3,875.11
	Mandor	Jam	0.056	Rp 10,655.82	Rp 595.70
				Jumlah	Rp 2,605,033.12

Tabel 4.40 Rencana anggaran biaya pelebaran jalan Bulu Tuban

No.	Uraian	Vol	Sat	Harga Satuan	Jumlah
1	Pekerjaan Persiapan				
	Mobilisasi	1	Ls	51600000	51.600.000.000
	Manajemen & Keselamatan Lalu-lintas	1	LS	66750000	66.750.000.000
					118.350.000.000
2	Pekerjaan Tanah				
	Galian Biasa	73125	m ³	121790.7973	8.905.952.049.378
					8.905.952.049.378
3	Pekerjaan Perkerasan dan Bahu Jalan				
	Lapis Pondasi Agregat Klas B (t : 6cm)	28125.5	m ³	515303.2221	14.493.160.773.206
	Lapis Pondasi Agregat Klas S (t : 6cm)	45000	m ³	254710.722	11.461.982.488.943
					25.955.143.262.149
4	Pekerjaan Perkerasan Berbutir				
	Lapis Pondasi atas bersemen (Cement Treated Base / C T B) (t : 6cm)	14062.5	m ³	516867.1814	7.268.444.738.626
					7.268.444.738.626
5	Pekerjaan Perkerasan Aspal				
	Lapis Perekat - Aspal Emulsi	30510	lt	11140.0938	339.884.261.829
	Split Matrik Aspal Modifikasi Crumb Rubber (SMA Mod. CR) (t : 5cm)	1695	m ³	2887280.841	4.893.941.024.714
	Laston Lapis Antara Modifikasi Crumb Rubber (AC-BC Mod.CR) (t : 6cm)	2034	m ³	2605033.115	5.298.637.356.444
					10.532.462.642.987
6	Pekerjaan Pemeliharaan				
	Pemeliharaan Rutin	1	Ls	5278035269	5.278.035.269.314

Tabel 4.41 Rekapitulasi rencana anggaran biaya pelebaran jalan Bulu Tuban

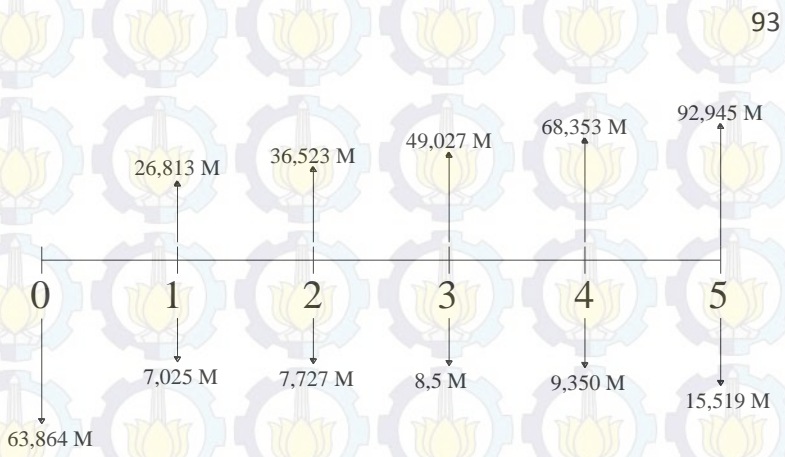
No.	Uraian	Harga
1	Pekerjaan Persiapan	118,350,000.000
2	Pekerjaan Tanah	8,905,952,049.378
3	Pekerjaan Perkerasan dan Bahu Jalan	25,955,143,262.149
4	Pekerjaan Perkerasan Berbutir	7,268,444,738.626
5	Pekerjaan Perkerasan Aspal	10,532,462,642.987
6	Pekerjaan Pemeliharaan	5,278,035,269.314
	Total	58,058,387,962.454
	PPN 10%	5,805,838,796.245
	Jumlah	63,864,226,758.699

4.8 Analisa Benefit Cost Ratio (BCR)

Benefit cost ratio merupakan salah satu metode untuk mengetahui kelayakan suatu proyek dari segi ekonomi. Pada dasarnya analisa perhitungan ini membandingkan antara besarnya biaya investasi yang dikeluarkan (cost) dengan besarnya biaya penghematan (benefit) yang diperoleh akibat proyek pelebaran jalan Bulu – Tuban tersebut.

Biaya konstruksi pelebaran jalan merupakan biaya investasi yang diperoleh dari perhitungan rencana anggaran biaya.

- Biaya konstruksi pelebaran jalan = Rp.63.864.226.758,699
- Benefit/Manfaat = penghematan BOK + penghematan nilai waktu
= 106.145.233.593,556 + 186.653.631.625,452
= Rp. 292.789.865.219,009
- Biaya pemeliharaan = 10% x Biaya konstruksi
= 10% x Rp. 63.864.226.758,699
= Rp. 6.386.422.675,869
- Biaya pemeliharaan 5 tahunan = Rp. 5.233.825.286,542



Gambar 4.11 Cash flow

Tabel 4.42 Tabel Perhitungan Present worth cost dan Present Worth Benefit

Tahun	n	Total biaya (cost)	Penghematan BOK (Rupiah/Tahun)	Penghematan Nilai Waktu (Rupiah/Tahun)	Penghematan Total (benefit) (Rupiah/Tahun)	(P/F, $i\%$,n) $i = 7.75\%$	Present worth cost	present worth benefit
		(Rupiah/Tahun)					(Rupiah/Tahun)	(Rupiah/Tahun)
2014	0	Rp 63,864,226,758.70				1	Rp 63,864,226,758.70	
2015	1	Rp 7,025,064,943.46	Rp 7,917,677,980.35	Rp 18,895,996,707.06	Rp 26,813,674,687.41	0.9281	Rp 6,519,781,850.08	Rp 24,885,080,916.39
2016	2	Rp 7,727,571,437.80	Rp 12,279,409,777.87	Rp 24,243,640,910.70	Rp 36,523,050,688.57	0.8613	Rp 6,655,925,786.62	Rp 31,458,099,978.85
2017	3	Rp 8,500,328,581.58	Rp 17,740,737,700.41	Rp 31,287,037,410.40	Rp 49,027,775,110.81	0.7994	Rp 6,794,912,635.99	Rp 39,191,361,300.65
2018	4	Rp 9,350,361,439.74	Rp 26,537,734,785.47	Rp 41,816,003,372.19	Rp 68,353,738,157.66	0.7419	Rp 6,936,801,762.96	Rp 50,709,946,820.01
2019	5	Rp 15,519,222,870.26	Rp 37,078,614,387.19	Rp 55,867,263,073.40	Rp 92,945,877,460.59	0.6885	Rp 10,685,222,645.22	Rp 63,994,660,230.41
						TOTAL	Rp 101,456,871,439.58	Rp 210,239,149,246.30
						BCR	2.072202171	
						NPV	Rp 108,782,277,806.73	

Present Worth Benefit = Rp. 210.239.149.246,30

Present Worth Cost = Rp. 101.456.871.439,58

$$\text{BCR} = \frac{\text{Rp.210.239.149.246,30}}{\text{Rp.101.456.871.439,58}} = 2,07 > 1$$

Dari hasil perhitungan diatas maka dapat disimpulkan proyek pelebaran jalan Bulu – Tuban layak dilaksanakan ditinjau dari sisi ekonomi.

4.9 Analisa Net Present Value (NPV)

Net Present Value (NPV) adalah sebagai salah satu parameter kelayakan selain analisis Benefit Cost Ratio (BCR) . Net Present Value (NPV) diperoleh dari selisih semua manfaat dengan semua pengeluaran. Nilai Net Present Value (NPV) akan menentukan layak tidaknya suatu proyek untuk dikerjakan. Suatu proyek layak untuk dikerjakan apabila nilai NPV= (+)

$$\begin{aligned} \text{NPV} &= \text{Present Worth Benefit} - \text{Present Worth Cost} \\ &= 210.239.149.246,30 - 101.456.871.439,58 \\ &= \text{Rp. } 108.782.277.806,73 > 0 \end{aligned}$$

Dari hasil perhitungan diatas didapatkan nilai NPV>0 maka dapat disimpulkan proyek pelebaran jalan Bulu – Tuban layak dilaksanakan ditinjau dari sisi ekonomi.

“Halaman ini sengaja dikosongkan”

BAB V

KESIMPULAN DAN SARAN

5.1 Kesimpulan

Dari hasil analisa perhitungan pada bab-bab sebelumnya maka dapat diambil kesimpulan sebagai berikut :

1. Kondisi karakteristik jalan Bulu Tuban sebelum dilakukan pelebaran adalah jalan 2 lajur 2 arah tak terbagi dengan $DS = 0,673$ dan $DB = 0,81$ pada tahun 2014, $DS = 0,969$ dan $DB = 0,9$ pada tahun 2019, sedangkan sesudah dilakukan pelebaran jalan, jalan Bulu Tuban menjadi 4 lajur 2 arah dengan $DS = 0,373$ pada tahun 2014 dan $DS = 0,879$ pada tahun 2034.
2. Nilai penghematan BOK pada jalan Bulu = Rp.106.145.233.593,556, penghematan nilai waktu = Rp.186.653.631.625,452, dan penghematan total = Rp.292.789.865.219,009
3. Dari segi ekonomi jumlah penghematan total (NPV) sebesar Rp. 108.782.277.806,73,- ($NPV > 0$) dan nilai BCR sebesar 2,07 ($BCR > 1$). Sehingga dari segi ekonomi proyek pelebaran jalan Bulu tuban dinyatakan layak dari segi ekonomi dikarenakan manfaat yang diterima lebih besar daripada biaya yang dikeluarkan.

5.2 Saran

Berdasarkan dari hasil analisa kelayakan pada proyek pelebaran jalan yang ditinjau dari segi ekonomi pelebaran jalan tersebut dinyatakan layak untuk dikerjakan. pada tahun 2030-2034 $DS > 0,75$ sehingga terjadi kemacetan. Untuk menghindari kemacetan pada tahun 2030 dan seterusnya sebaiknya dilakukan pelebaran jalan.

KAJI -- INTERURBAN ROADS Province: JAWA TIMUR Date: JUNI 2014
 Link number: ARIF
 Form IR-2: Input Segment code: Checked by: ARIF
 TRAFFIC FLOW, SIDE FRICTION Administr. road class : NATIONAL Functional road class: ARTERIAL
 Road type : 2/2UD Length (km) : 7.500
 Purpose: Operation Time period : Case number:

TRAFFIC DATA:

Type of traffic data	ANNUAL AVERAGE DAILY TRAFFIC		DIRECTIONAL SPLIT
CLASSIFIED-HOURLY	AADT	K-factor	Dir1 - Dir2
(Class/Aadt/UNclass)	(veh/day)	(default: 0.11)	(default: 50 - 50)
			NA - NA %

Traffic Composition(%)	LV (%)	MHV (%)	LB (%)	LT (%)	MC (%)	Total (%)	LV = Light Vehicle
User values	31.05	34.42	0.197	3.412	30.90	100.0	MHV = Medium Heavy Vehicle
(normal values)	(57.0)	(23.0)	(7.0)	(4.0)	(9.0)	(100.0)	LB = Large Bus
							LT = Large Truck
							MC = MotorCycle

Traffic flow data for whole segment analysis:

Row	Dir	Light Vehicle	Med Heavy Veh	Large Bus	Large Truck	MotorCycle	Total flow Q	
1.1	Dir1	pce,1= 1.00	pce,1= 1.30	pce,1= 1.50	pce,1= 2.50	pce,1= 0.50		
1.2	Dir2	pce,2= 1.00	pce,2= 1.30	pce,2= 1.50	pce,2= 2.50	pce,2= 0.50		
		veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	Split veh/h pcu/h	
2	(1)	(2) (3)	(4) (5)	(6) (7)	(8) (9)	(10) (11)	(%) (12) (13) (14)	
3	Dir1	322	322	428	556	2	3	
4	Dir2	306	306	268	348	2	3	
5	1+2	628	628	696	904	4	6	
6	Note.	If specific grade then				Directional split. SP= Q1/(Q1+Q2)=		54.9%
7	dir 1 = uphill, dir 2= downhill					Pcu-factor, Fpcu =		56.1%
								1.000

SIDE FRICTION CLASS: If detailed data are available, use first table to determine weighted frequency of events and then go to second table. If not, use second table only.

1. Determination of frequency of events

Calculation of weighted frequency of events per hour and 200 m of the studied road segment.	Side friction type of events (20)	Symbol	Weighting factor (22)	Frequency of events (23)	Weighted frequency (24)
Frequencies are for both sides of the road.	Pedestrians	PED	0.6	NA / h,200m	NA
	Parking, stopping veh.	PSV	0.8	NA / h,200m	NA
	Entry+exit of vehicles	EEV	1.0	NA / h,200m	NA
	Slow-moving vehicles	SMV	0.4	NA / h	NA
				Total:	NA

2. Determination of side friction class

Weighted frequency of events (30)	Typical conditions	Side friction class
< 50	Rural, agriculture or undeveloped with very few activities	VL= very low
50 - 149	Rural, some roadside buildings and some activities	L= low
150 - 249	Village, residential activities	M= medium
250 - 349	Village, some market activities	H= high
> 350	Almost urban, market and business activities	VH= very high
For current case indicate side friction class: L (L is default)		

KAJI -- INTERURBAN ROADS		Province:	JAWA TIMUR			Date:	JUNI 2014							
Form IR-3: Analysis		Link number:				Handled by:	ARIF							
		Segment code:				Checked by:	ARIF							
SPEED, CAPACITY		Administr. road class :	NATIONAL			Functional road class:	ARTERIAL							
Purpose: Operation		Road type :	2/2UD			Length (km)	7.500							
		Time period :				Case number:								
FREE FLOW SPEEDS														
Option to enter other free flow speeds: No														
Di- rec- tion	Base free-flow speed FVo (km/h)					Carriage-way width adjust-ment, FVw		FVo+FVw Light	Adjustment factors		Actual free-flow speeds, km/h			
	Table B-1:1 or B-1:2					Tab B2:1		(2)+(3)	Side friction FVvsc	Land use	FFVlv = (FVo+FVw)*FFVsc*FFVrc			
	LV	MHV	LB	LT	MC	(km/h)	(km/h)	Tab B3:1	Tab B4:1	Light	Other vehicle types			
(2)	(2)	(2)	(2)	(2)	(3)	(4)	(5)	(6)	(7)	MHV	LB	LT	MC	
1+2	68.0	60.0	73.0	58.0	55.0	0.0	68.0	0.980	1.000	66.64	58.80	71.54	56.84	53.90
Comments:										User FFV, dir1: None! dir2:				
CAPACITY														
Direc- tion	Base Capacity		Adjustment factors for capacity					Actual capacity, C						
	Co	Carriageway width	Directional split	Side friction	C= Co*FCw*FCsp*FCsf			pcu/h						
Table C-1:1	Table C-2:1	Table C-3:1	Table C-4:1	(11)*(12)*(13)*(14)			(15)							
(11)	(12)	(13)	(14)	(11)			(15)							
1+2	3100	1.000	0.970	1.000	3008									
ACTUAL SPEED and TRAVEL TIME for light vehicles										Only 2/2UD roads				
Di- rec- tion	Traffic flow, Q	Degree of saturation	Actual speed, Vlv	Road segment length, L	Travel time, TT	ACTUAL SPEEDS				Di- rec- tion	Degree of			
	Form IR-2	DS=Q/C	Fig D2:1/:2	km	sec	for other vehicle types					lunching			
	pcu/h	(21)/(15)	Km/h	km	sec	km/h					DB			
(21)	(22)	(23)	(24)	(25)		MHV	LB	LT	MC	Fig D3:1	(31)			
1+2	2024	0.673	43.58	7.500	619.433	40.70	45.38	39.98	38.90	1+2	0.809			
Space for user remark:														
Program version 1.10F Date of run: 150121/15:33														

K A J I	Province	JAWA TIMUR	Date	: JUNI 2014																														
INTERURBAN ROADS	Link number:		Handled by :	ARIF																														
	Segment code:		Checked by :	ARIF																														
Form IR-1: Input	Segment between	BULU and	TUBAN																															
	Specific grade:	No [NO indicates segment, YES spec grade(only 2/2UD)]																																
GENERAL DATA,	Administr. road class :	NATIONAL	Functional road class:	ARTERIAL																														
ROAD GEOMETRY	Road type :	2/2UD	Length (km) :	7.500																														
Purpose: Operation	Time period:		Case number:																															
HORIZONTAL ALIGNMENT																																		
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<pre> Undivided road side A ##### ##### side B WsA WcA WcB WsB 1.50 3.50 3.50 1.50 </pre> <table border="1"> <tr> <td>UNADJUSTED WIDTHS</td> <td>Side A</td> <td>Side B</td> <td>Total</td> <td>Mean</td> </tr> <tr> <td>Average carriageway width, Wc (m)</td> <td>3.50</td> <td>3.50</td> <td>7.00</td> <td></td> </tr> <tr> <td>Unobstructed shoulder width, Ws (m)</td> <td>1.50</td> <td>1.50</td> <td>3.00</td> <td>1.50</td> </tr> </table>					UNADJUSTED WIDTHS	Side A	Side B	Total	Mean	Average carriageway width, Wc (m)	3.50	3.50	7.00		Unobstructed shoulder width, Ws (m)	1.50	1.50	3.00	1.50															
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SHOULDER SURFACE CONDITIONS	SIDE A		SIDE B																															
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Program version 1.10F Date of run: 140610/6:24																																		

KAJI -- INTERURBAN ROADS Province: JAWA TIMUR Date: JUNI 2014
 Link number: ARIF
 Form IR-2: Input Segment code: Checked by: ARIF
 TRAFFIC FLOW, SIDE FRICTION Administr. road class : NATIONAL Functional road class: ARTERIAL
 Road type : 2/2UD Length (km) : 7.500
 Purpose: Operation Time period : Case number:

TRAFFIC DATA:

Type of traffic data	ANNUAL AVERAGE DAILY TRAFFIC				DIRECTIONAL SPLIT	
CLASSIFIED-HOURLY	ADDT (veh/day)	K-factor (default: 0.11)			Dir1 - Dir2 (default: 50 - 50)	
(Class/Aadt/UNclass)					NA - NA %	

Traffic Composition(%)	LV (%)	MHV (%)	LB (%)	LT (%)	MC (%)	Total (%)	LV = Light Vehicle
User values	31.17	34.34	0.183	3.484	30.81	100.0	MHV = Medium Heavy Vehicle
(normal values)	(57.0)	(23.0)	(7.0)	(4.0)	(9.0)	(100.0)	LB = Large Bus
							LT = Large Truck
							MC = MotorCycle

Traffic flow data for whole segment analysis:

Row	Dir	Light Vehicle	Med Heavy Veh	Large Bus	Large Truck	MotorCycle	Total flow Q	
1.1	Dir1	pce,1= 1.00	pce,1= 1.30	pce,1= 1.50	pce,1= 2.50	pce,1= 0.50		
1.2	Dir2	pce,2= 1.00	pce,2= 1.30	pce,2= 1.50	pce,2= 2.50	pce,2= 0.50		
		veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	Split veh/h pcu/h	
2	(1)	(2) (3)	(4) (5)	(6) (7)	(8) (9)	(10) (11)	(12) (13) (14)	
3	Dir1	349	349	460	598	2	3	
4	Dir2	331	331	289	376	2	3	
5	1+2	680	680	749	974	4	6	
6	Note.	If specific grade then				Directional split. SP= Q1/(Q1+Q2)=		54.9% 56.1%
7		dir 1 = uphill, dir 2= downhill				Pcu-factor, Fpcu =		1.002

SIDE FRICTION CLASS: If detailed data are available, use first table to determine weighted frequency of events and then go to second table. If not, use second table only.

1. Determination of frequency of events

Calculation of weighted frequency of events per hour and 200 m of the studied road segment.	Side friction type of events (20)	Symbol	Weighting factor (22)	Frequency of events (23)	Weighted frequency (24)
Frequencies are for both sides of the road.	Pedestrians	PED	0.6	NA / h,200m	NA
	Parking, stopping veh.	PSV	0.8	NA / h,200m	NA
	Entry+exit of vehicles	EEV	1.0	NA / h,200m	NA
	Slow-moving vehicles	SMV	0.4	NA / h	NA
Total:					NA

2. Determination of side friction class

Weighted frequency of events (30)	Typical conditions	Side friction class
< 50	Rural, agriculture or undeveloped with very few activities	VL= very low
50 - 149	Rural, some roadside buildings and some activities	L= low
150 - 249	Village, residential activities	M= medium
250 - 349	Village, some market activities	H= high
> 350	Almost urban, market and business activities	VH= very high
For current case indicate side friction class: L (L is default)		

KAJI -- INTERURBAN ROADS		Province:	JAWA TIMUR		Date:	JUNI 2014								
Form IR-3: Analysis		Link number:			Handled by:	ARIF								
		Segment code:			Checked by:	ARIF								
SPEED, CAPACITY		Administr. road class :	NATIONAL		Functional road class:	ARTERIAL								
Purpose: Operation		Road type :	2/2UD		Length (km) :	7.500								
		Time period :			Case number:									
FREE FLOW SPEEDS														
Option to enter other free flow speeds: No														
Di- rec- tion	Base free-flow speed FVo (km/h)		Carriage- way width adjust- ment, FVw Tab E2:1	FVo+FVw Light	Adjustment factors		Actual free-flow speeds, km/h							
	for different vehicles Table B-1:1 or B-1:2			Side friction FFVsf	Land use FFVrc	FFVlv = (FVo+FVw)*FFVsf*FFVrc		Other vehicle types						
	LV	MHV	LB	LT	MC	(2)+(3)	Tab B3:1	Tab B4:1	(4)*(5)*(6)	MHV	LB	LT	MC	
	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(13)	(14)	(15)
1+2	68.0	60.0	73.0	58.0	55.0	0.0	68.0	0.970	1.000	65.96	58.20	70.81	56.26	53.35
Comments:					User FFV, dir1: None!			dir2:						
CAPACITY														
Direc- tion	Base Capacity		Adjustment factors for capacity				Actual capacity, C							
	Co pcu/h Table C-1:1 (11)	Carriageway width FCw Table C-2:1 (12)	Directional split FCsp Table C-3:1 (13)	Side friction FCsf Table C-4:1 (14)	C= Co*FCw*FCsp*FCsf pcu/h (11)*(12)*(13)*(14) (15)									
1+2	3100	1.000	0.970	0.970	2918									
ACTUAL SPEED and TRAVEL TIME for light vehicles								Only 2/2UD roads						
Di- rec- tion	Traffic flow, Q	Degree of saturation	Actual speed, Vlv	Road segment length, L	Travel time, TT	ACTUAL SPEEDS				Di- rec- tion	Degree of bunching DB			
	Form IR-2 pcu/h (21)	DS=Q/C (21)/(15) (22)	Fig D2:1/:2 km/h (23)	km (24)	sec (25)	for other vehicle types km/h					Fig D3:1 (31)			
1+2	2187	0.749	40.58	7.500	665.298	38.28	42.01	37.71	36.85	1+2	0.838			
Space for user remark:														
Program version 1.10F Date of run: 140610/6:24														

K A J I	Province	JAWA TIMUR	Date	JUNI 2014																														
INTERURBAN ROADS	Link number:		Handled by :	ARIF																														
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<pre> To: <----- * +--> A * * * * * +-----> To: TUBAN BULU * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * </pre>																																		
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Program version 1.10F Date of run: 140610/6:32																																		

KAJI -- INTERURBAN ROADS Province: JAWA TIMUR Date: JUNI 2014
 Link number: ARIF
 Form IR-2: Input Segment code: Checked by: ARIF
 TRAFFIC FLOW, SIDE FRICTION Administr. road class : NATIONAL Functional road class: ARTERIAL
 Road type : 2/2UD Length (km) : 7.500
 Purpose: Operation Time period : Case number:

TRAFFIC DATA:

Type of traffic data	ANNUAL AVERAGE DAILY TRAFFIC		DIRECTIONAL SPLIT	
CLASSIFIED-HOURLY	AADT	K-factor	Dir1 - Dir2	
(Class/Aadt/UNclass)	(veh/day)	(default: 0.11)	(default: 50 - 50)	
			NA - NA %	

Traffic Composition(%)	LV (%)	MHV (%)	LB (%)	LT (%)	MC (%)	Total (%)	LV = Light Vehicle
User values	31.33	34.28	0.171	3.463	30.73	100.0	MHV = Medium Heavy Vehicle
(normal values)	(57.0)	(23.0)	(7.0)	(4.0)	(9.0)	(100.0)	LB = Large Bus
							LT = Large Truck
							MC = MotorCycle

Traffic flow data for whole segment analysis:

Row	Dir	Light Vehicle	Med Heavy Veh	Large Bus	Large Truck	MotorCycle	Total flow Q	
1.1	Dir1	pce,1= 1.00	pce,1= 1.30	pce,1= 1.50	pce,1= 2.50	pce,1= 0.50		
1.2	Dir2	pce,2= 1.00	pce,2= 1.30	pce,2= 1.50	pce,2= 2.50	pce,2= 0.50		
		veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	Split veh/h pcu/h (%)	
2	(1)	(2) (3)	(4) (5)	(6) (7)	(8) (9)	(10) (11)	(12) (13) (14)	
3	Dir1	376	376	493	641	2	3	
4	Dir2	357	357	309	402	2	3	
5	1+2	733	733	802	1043	4	6	
6	Note.	If specific grade then				Directional split. SP= Q1/(Q1+Q2)=		54.9% 56.2%
7	dir 1 = uphill, dir 2= downhill					Pcu-factor, Fpcu =		1.002

SIDE FRICTION CLASS: If detailed data are available, use first table to determine weighted frequency of events and then go to second table. If not, use second table only.

1. Determination of frequency of events

Calculation of weighted frequency of events per hour and 200 m of the studied road segment.	Side friction type of events (20)	Symbol	Weighting factor (22)	Frequency of events (23)	Weighted frequency (24)
Frequencies are for both sides of the road.	Pedestrians	PED	0.6	NA / h,200m	NA
	Parking, stopping veh.	PSV	0.8	NA / h,200m	NA
	Entry+exit of vehicles	EEV	1.0	NA / h,200m	NA
	Slow-moving vehicles	SMV	0.4	NA / h	NA
Total:					NA

2. Determination of side friction class

Weighted frequency of events (30)	Typical conditions	Side friction class
< 50	Rural, agriculture or undeveloped with very few activities	VL= very low
50 - 149	Rural, some roadside buildings and some activities	L= low
150 - 249	Village, residential activities	M= medium
250 - 349	Village, some market activities	H= high
> 350	Almost urban, market and business activities	VH= very high
For current case indicate side friction class: L (L is default)		

KAJI -- INTERURBAN ROADS		Province:	JAWA TIMUR		Date:	JUNI 2014								
Form IR-3: Analysis		Link number:			Handled by:	ARIF								
		Segment code:			Checked by:	ARIF								
SPEED, CAPACITY		Administr. road class :	NATIONAL		Functional road class:	ARTERIAL								
Purpose: Operation		Road type :	2/2UD		Length (km)	7.500								
		Time period :			Case number:									
FREE FLOW SPEEDS														
Option to enter other free flow speeds: No														
Di- rec- tion	Base free-flow speed FVo (km/h)		Carriage- way width adjust- ment, FVw Tab E2:1	FVo+FVw Light	Adjustment factors		Actual free-flow speeds, km/h							
	for different vehicles Table B-1:1 or B-1:2			vehicle	Side friction	Land use	FFVlv = (FVo+FVw)*FFVsf*FFVrc							
	LV	MHV	LB	LT	MC	(2)+(3)	FFVsf	FFVrc	Light	Other vehicle types				
	(2)	(2)	(2)	(2)	(2)	(2)	Tab B3:1	Tab B4:1	(4)*(5)*(6)	MHV	LB	LT	MC	
1+2	68.0	60.0	73.0	58.0	55.0	0.0	68.0	0.970	1.000	65.96	58.20	70.81	56.26	53.35
Comments:								User FFV, dir1: None! dir2:						
CAPACITY														
Direc- tion	Base Capacity		Adjustment factors for capacity				Actual capacity, C							
	Co pcu/h Table C-1:1 (11)	Carriageway width FCw Table C-2:1 (12)	Directional split FCsp Table C-3:1 (13)	Side friction FCsf Table C-4:1 (14)	C= Co*FCw*FCsp*FCsf pcu/h (11)*(12)*(13)*(14) (15)									
1+2	3100	1.000	0.970	0.970	2918									
ACTUAL SPEED and TRAVEL TIME for light vehicles								Only 2/2UD roads						
Di- rec- tion	Traffic flow, Q	Degree of saturation DS=Q/C	Actual speed, Vlv Fig D2:1/:2	Road segment length, L	Travel time, TT (24/23)	ACTUAL SPEEDS for other vehicle types km/h				Di- rec- tion	Degree of bunching DB Fig D3:1			
	pcu/h (21)	(21)/(15) (22)	Km/h (23)	km (24)	sec (25)	MHV	LB	LT	MC		(31)			
	1+2	2345	0.804	38.74	7.500	696.775	36.84	39.93	36.37	35.66	1+2	0.856		
Space for user remark:														
Program version 1.10F								Date of run: 140610/6:32						

KAJI -- INTERURBAN ROADS Province: JAWA TIMUR Date: JUNI 2014
 Link number: ARIF
 Form IR-2: Input Segment code: Checked by: ARIF
 TRAFFIC FLOW, SIDE FRICTION Administr. road class : NATIONAL Functional road class: ARTERIAL
 Road type : 2/2UD Length (km) : 7.500
 Purpose: Operation Time period : Case number:

TRAFFIC DATA

Type of traffic data	ANNUAL AVERAGE DAILY TRAFFIC		DIRECTIONAL SPLIT	
CLASSIFIED-HOURLY	AADT	K-factor	Dir1 - Dir2	
(Class/Aadt/UNclass)	(veh/day)	(default: 0.11)	(default: 50 - 50)	
			NA - NA %	

Traffic Composition(%)	LV (%)	MHV (%)	LB (%)	LT (%)	MC (%)	Total (%)	LV = Light Vehicle
User values	31.42	34.26	0.160	3.522	30.62	100.0	MHV = Medium Heavy Vehicle
(normal values)	(57.0)	(23.0)	(7.0)	(4.0)	(9.0)	(100.0)	LB = Large Bus
							LT = Large Truck
							MC = MotorCycle

Traffic flow data for whole segment analysis:

Row	Dir	Light Vehicle	Med Heavy Veh	Large Bus	Large Truck	MotorCycle	Total flow Q	
1.1	Dir1	pce,1= 1.00	pce,1= 1.30	pce,1= 1.50	pce,1= 2.50	pce,1= 0.50		
1.2	Dir2	pce,2= 1.00	pce,2= 1.30	pce,2= 1.50	pce,2= 2.50	pce,2= 0.50		
		veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	Split veh/h pcu/h	
2	(1)	(2) (3)	(4) (5)	(6) (7)	(8) (9)	(10) (11)	(%) (12) (13) (14)	
3	Dir1	403	403	526	684	2	3	
4	Dir2	382	382	330	429	2	3	
5	1+2	785	785	856	1113	4	6	
6	Note. If specific grade then					Directional split. SP= Q1/(Q1+Q2)=		54.9% 56.2%
7	dir 1 = uphill, dir 2= downhill					Pcu-factor, Fpcu =		1.004

SIDE FRICTION CLASS: If detailed data are available, use first table to determine weighted frequency of events and then go to second table. If not, use second table only.

1. Determination of frequency of events

Calculation of weighted frequency of events per hour and 200 m of the studied road segment.	Side friction type of events (20)	Symbol	Weighting factor (22)	Frequency of events (23)	Weighted frequency (24)
Frequencies are for both sides of the road.	Pedestrians	PED	0.6	NA / h,200m	NA
	Parking, stopping veh.	PSV	0.8	NA / h,200m	NA
	Entry+exit of vehicles	EEV	1.0	NA / h,200m	NA
	Slow-moving vehicles	SMV	0.4	NA / h	NA
Total:					NA

2. Determination of side friction class

Weighted frequency of events (30)	Typical conditions	Side friction class
< 50	Rural, agriculture or undeveloped with very few activities	VL= very low
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For current case indicate side friction class: L (L is default)		

KAJI -- INTERURBAN ROADS		Province:	JAWA TIMUR			Date:	JUNI 2014								
Form IR-3: Analysis		Link number:				Handled by:	ARIF								
		Segment code:				Checked by:	ARIF								
SPEED, CAPACITY		Administr. road class :	NATIONAL			Functional road class:	ARTERIAL								
Purpose: Operation		Road type :	2/2UD			Length (km)	7.500								
		Time period :				Case number:									
FREE FLOW SPEEDS															
Option to enter other free flow speeds: No															
Di- rec- tion	Base free-flow speed FVo (km/h)					Carriage-way width adjust-ment, FVw		FVo+FVw Light	Adjustment factors		Actual free-flow speeds, km/h				
	Table B-1:1 or B-1:2					Tab B2:1		(2)+(3)	Side friction FVvsc	Land use	Light	Other vehicle types			
	LV	MHV	LB	LT	MC	(km/h)	(km/h)	Tab B3:1	Tab B4:1	(4*5*6)		MHV	LB	LT	MC
	(2)					(3)	(4)	(5)	(6)	(7)					
1+2	68.0	60.0	73.0	58.0	55.0	0.0	68.0	0.970	1.000	65.96	58.20	70.81	56.26	53.35	
Comments:										User FFV, dir1: None! dir2:					
CAPACITY															
Direc- tion	Base Capacity		Adjustment factors for capacity					Actual capacity, C							
	Co		Carriageway width		Directional split		Side friction		C= Co*FCw*FCsp*FCsf pcu/h						
	Table C-1:1		Table C-2:1		Table C-3:1		Table C-4:1		(11)*(12)*(13)*(14)						
	pcu/h														
	(11)		(12)		(13)		(14)							(15)	
1+2	3100		1.000		0.970		0.970							2917	
ACTUAL SPEED and TRAVEL TIME for light vehicles										Only 2/2UD roads					
Di- rec- tion	Traffic flow, Q	Degree of saturation	Actual speed, Vlv	Road segment length, L	Travel time, TT	ACTUAL SPEEDS				Di- rec- tion	Degree of bunching DB				
	Form IR-2	DS=Q/C	Fig D2:1/:2	km	sec	for other vehicle types									
	pcu/h	(21)/(15)	Km/h	km	sec	MHV	LB	LT	MC			Fig D3:1			
	(21)	(22)	(23)	(24)	(25)					(31)					
1+2	2508	0.860	36.70	7.500	735.563	35.24	37.61	34.88	34.33	1+2	0.873				
Space for user remark:															
Program version 1.10F Date of run: 140610/6:35															

K A J I	Province	JAWA TIMUR	Date	: JUNI 2014																														
INTERURBAN ROADS	Link number:		Handled by :	ARIF																														
	Segment code:		Checked by :	ARIF																														
Form IR-1: Input	Segment between	BULU and	TUBAN																															
	Specific grade:	No [NO indicates segment, YES spec grade(only 2/2UD)]																																
GENERAL DATA,	Administr. road class :	NATIONAL	Functional road class:	ARTERIAL																														
ROAD GEOMETRY	Road type :	2/2UD	Length (km) :	7.500																														
Purpose: Operation	Time period:		Case number:																															
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<pre> To: <----- * +--> A * * * * * +-----> To: TUBAN BULU * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * </pre>																																		
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TRAFFIC CONTROL CONDITIONS																																		
<table border="1"> <tr> <td>Speed limit</td> <td>: NA km/h</td> <td>Max gross weight:</td> <td>0.000 tonnes</td> </tr> <tr> <td>Other limitations</td> <td>:</td> <td></td> <td></td> </tr> <tr> <td>More remarks</td> <td>:</td> <td></td> <td></td> </tr> </table>					Speed limit	: NA km/h	Max gross weight:	0.000 tonnes	Other limitations	:			More remarks	:																				
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Other limitations	:																																	
More remarks	:																																	
Program version 1.10F Date of run: 140610/6:37																																		

KAJI -- INTERURBAN ROADS	Province: JAWA TIMUR	Date: JUNI 2014
Form IR-2: Input	Link number:	Handled by: ARIF
	Segment code:	Checked by: ARIF
TRAFFIC FLOW, SIDE FRICTION	Administr. road class : NATIONAL	Functional road class: ARTERIAL
	Road type : 2/2UD	Length (km) : 7.500
Purpose: Operation	Time period :	Case number:

TRAFFIC DATA:									
Type of traffic data		ANNUAL AVERAGE DAILY TRAFFIC					DIRECTIONAL SPLIT		
CLASSIFIED-HOURLY		AADT		K-factor			Dir1 - Dir2		
(Class/Aadt/UNclass)		(veh/day)		(default: 0.11)			(default: 50 - 50)		
							NA - NA %		
Traffic Composition(%)	LV (%)	MHV (%)	LB (%)	LT (%)	MC (%)	Total (%)	LV = Light Vehicle	MHV = Medium Heavy Vehicle	LB = Large Bus
User values	31.51	34.22	0.150	3.539	30.57	100.0	LT = Large Truck	MC = MotorCycle	
(normal values)	(57.0)	(23.0)	(7.0)	(4.0)	(9.0)	(100.0)			

Traffic flow data for whole segment analysis:

Row	Dir	Light Vehicle	Med Heavy Veh	Large Bus	Large Truck	MotorCycle	Total flow Q								
1.1	tion	pce,1= 1.00	pce,1= 1.30	pce,1= 1.50	pce,1= 2.50	pce,1= 0.50									
1.2		pce,2= 1.00	pce,2= 1.30	pce,2= 1.50	pce,2= 2.50	pce,2= 0.50									
		veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	Split (%)	veh/h pcu/h							
2	(1)	(2) (3)	(4) (5)	(6) (7)	(8) (9)	(10) (11)	(12)	(13) (14)							
3	Dir1	430	430	558	725	2	3	52	130	417	209	54.93	1459	1497	
4	Dir2	407	407	351	456	2	3	42	105	395	198	45.06	1197	1169	
5	1+2	837	837	909	1181	4	6	94	235	812	407		2656	2666	
6	Note.	If specific grade then					Directional split. SP= Q1/(Q1+Q2)=		54.9%	56.1%					
7		dir 1 = uphill, dir 2= downhill					Pcu-factor, Fpcu =								1.003

SIDE FRICTION CLASS: If detailed data are available, use first table to determine weighted frequency of events and then go to second table. If not, use second table only.

1. Determination of frequency of events

Calculation of weighted frequency of events per hour and 200 m of the studied road segment.	Side friction type of events (20)	Symbol	Weighting factor (22)	Frequency of events (23)	Weighted frequency (24)
Frequencies are for both sides of the road.	Pedestrians	PED	0.6	NA / h,200m	NA
	Parking, stopping veh.	PSV	0.8	NA / h,200m	NA
	Entry+exit of vehicles	EEV	1.0	NA / h,200m	NA
	Slow-moving vehicles	SMV	0.4	NA / h	NA
Total:					NA

2. Determination of side friction class

Weighted frequency of events (30)	Typical conditions	Side friction class
< 50	Rural, agriculture or undeveloped with very few activities	VL= very low
50 - 149	Rural, some roadside buildings and some activities	L= low
150 - 249	Village, residential activities	M= medium
250 - 349	Village, some market activities	H= high
> 350	Almost urban, market and business activities	VH= very high
For current case indicate side friction class: L (L is default)		

KAJI -- INTERURBAN ROADS		Province:	JAWA TIMUR	Date:	JUNI 2014
Form IR-3: Analysis		Link number:		Handled by:	ARIF
		Segment code:		Checked by:	ARIF
SPEED, CAPACITY		Administr. road class :	NATIONAL	Functional road class:	ARTERIAL
Purpose: Operation		Road type :	2/2UD	Length (km) :	7.500
		Time period :		Case number:	

FREE FLOW SPEEDS
Option to enter other free flow speeds: No

Di- rec- tion	Base free-flow speed FVo (km/h) for different vehicles					Carriage- way width adjust- ment, FVw Tab E2:1 (km/h) (3)	FVo+FVw Light (2)+(3) (km/h) (4)	Adjustment factors			Actual free-flow speeds, km/h FFVlv = (FVo+FVw)*FFVsf*FFVrc				
	LV (2)	MHV (2)	LB	LT	MC			Side friction FFVsf Tab B3:1 (5)	Land use FFVrc Tab B4:1 (6)	Light (4*5*6) (7)	Other vehicle types				
1+2	68.0	60.0	73.0	58.0	55.0	0.0	68.0	0.970	1.000	65.96	58.20	70.81	56.26	53.35	

Comments: User FFV, dir1: None; dir2:

Direc- tion	Base Capacity		Adjustment factors for capacity				Actual capacity, C	
	Co pcu/h Table C-1:1 (11)	Carriageway width FCw Table C-2:1 (12)	Directional split FCsp Table C-3:1 (13)	Side friction FCsf Table C-4:1 (14)	C= Co*FCw*FCsp*FCsf (11)*(12)*(13)*(14) (15)			
1+2	3100	1.000	0.970	0.970	2918			

ACTUAL SPEED and TRAVEL TIME for light vehicles										Only 2/2UD roads	
Di- rec- tion	Traffic flow, Q Form IR-2 pcu/h (21)	Degree of saturation DS=Q/C (21)/(15) (22)	Actual speed, Vlv Fig D2:1/:2 km/h (23)	Road segment length, L km (24)	Travel time, TT (24/23) sec (25)	ACTUAL SPEEDS for other vehicle types km/h				Di- rec- tion	Degree of bunching DB Fig D3:1 (31)
						MHV	LB	LT	MC		
1+2	2666	0.914	34.10	7.500	791.749	33.20	34.66	32.97	32.63	1+2	0.888

Space for user remark:

Program version 1.10F | Date of run: 140610/6:37 |

K A J I	Province	JAWA TIMUR	Date	: JUNI 2014																														
INTERURBAN ROADS	Link number:		Handled by :	ARIF																														
	Segment code:		Checked by :	ARIF																														
Form IR-1: Input	Segment between	BULU and	TUBAN																															
	Specific grade:	No [NO indicates segment, YES spec grade(only 2/2UD)]																																
GENERAL DATA,	Administr. road class :	NATIONAL	Functional road class:	ARTERIAL																														
ROAD GEOMETRY	Road type :	2/2UD	Length (km) :	7.500																														
Purpose: Operation	Time period:		Case number:																															
HORIZONTAL ALIGNMENT																																		
<pre> To: <----- * +--> A * * * * * +-----> To: TUBAN BULU * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * </pre>																																		
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<pre> Undivided road side A ##### ##### side B side A WsA WcA WcB WsB side B 1.50 3.50 3.50 1.50 </pre> <table border="1"> <tr> <td>UNADJUSTED WIDTHS</td> <td>Side A</td> <td>Side B</td> <td>Total</td> <td>Mean</td> </tr> <tr> <td>Average carriageway width, Wc (m)</td> <td>3.50</td> <td>3.50</td> <td>7.00</td> <td></td> </tr> <tr> <td>Unobstructed shoulder width, Ws (m)</td> <td>1.50</td> <td>1.50</td> <td>3.00</td> <td>1.50</td> </tr> </table>					UNADJUSTED WIDTHS	Side A	Side B	Total	Mean	Average carriageway width, Wc (m)	3.50	3.50	7.00		Unobstructed shoulder width, Ws (m)	1.50	1.50	3.00	1.50															
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SHOULDER SURFACE CONDITIONS	SIDE A		SIDE B																															
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Program version 1.10F Date of run: 140610/6:39																																		

KAJI -- INTERURBAN ROADS Province: JAWA TIMUR Date: JUNI 2014
 Link number: ARIF
 Form IR-2: Input Segment code: Checked by: ARIF
 TRAFFIC FLOW, SIDE FRICTION Administr. road class : NATIONAL Functional road class: ARTERIAL
 Road type : 2/2UD Length (km) : 7.500
 Purpose: Operation Time period : Case number:

TRAFFIC DATA:

Type of traffic data	ANNUAL AVERAGE DAILY TRAFFIC		DIRECTIONAL SPLIT
CLASSIFIED-HOURLY	AADT	K-factor	Dir1 - Dir2
(Class/Aadt/UNclass)	(veh/day)	(default: 0.11)	(default: 50 - 50)
			NA - NA %

Traffic Composition(%)	LV (%)	MHV (%)	LB (%)	LT (%)	MC (%)	Total (%)	LV = Light Vehicle
User values	31.58	34.17	0.142	3.587	30.51	100.0	MHV = Medium Heavy Vehicle
(normal values)	(57.0)	(23.0)	(7.0)	(4.0)	(9.0)	(100.0)	LB = Large Bus
							LT = Large Truck
							MC = MotorCycle

Traffic flow data for whole segment analysis:

Row	Dir	Light Vehicle	Med Heavy Veh	Large Bus	Large Truck	MotorCycle	Total flow Q	
1.1	Dir1	pce,1= 1.00	pce,1= 1.30	pce,1= 1.50	pce,1= 2.50	pce,1= 0.50		
1.2	Dir2	pce,2= 1.00	pce,2= 1.30	pce,2= 1.50	pce,2= 2.50	pce,2= 0.50		
		veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	Split veh/h pcu/h	
2	(1)	(2) (3)	(4) (5)	(6) (7)	(8) (9)	(10) (11)	(%) (12) (13) (14)	
3	Dir1	457	457	591	768	2	3	
4	Dir2	432	432	371	482	2	3	
5	1+2	889	889	962	1250	4	6	
6	Note.	If specific grade then				Directional split. SP= Q1/(Q1+Q2)=		54.9% 56.1%
7	dir 1 = uphill, dir 2= downhill					Pcu-factor, Fpcu =		1.004

SIDE FRICTION CLASS: If detailed data are available, use first table to determine weighted frequency of events and then go to second table. If not, use second table only.

1. Determination of frequency of events

Calculation of weighted frequency of events per hour and 200 m of the studied road segment.	Side friction type of events (20)	Symbol	Weighting factor (22)	Frequency of events (23)	Weighted frequency (24)
Frequencies are for both sides of the road.	Pedestrians	PED	0.6	NA / h,200m	NA
	Parking, stopping veh.	PSV	0.8	NA / h,200m	NA
	Entry+exit of vehicles	EEV	1.0	NA / h,200m	NA
	Slow-moving vehicles	SMV	0.4	NA / h	NA
Total:					NA

2. Determination of side friction class

Weighted frequency of events (30)	Typical conditions	Side friction class
< 50	Rural, agriculture or undeveloped with very few activities	VL= very low
50 - 149	Rural, some roadside buildings and some activities	L= low
150 - 249	Village, residential activities	M= medium
250 - 349	Village, some market activities	H= high
> 350	Almost urban, market and business activities	VH= very high
For current case indicate side friction class: L (L is default)		

KAJI -- INTERURBAN ROADS		Province:	JAWA TIMUR			Date:	JUNI 2014								
Form IR-3: Analysis		Link number:				Handled by:	ARIF								
		Segment code:				Checked by:	ARIF								
SPEED, CAPACITY		Administr. road class :	NATIONAL			Functional road class:	ARTERIAL								
Purpose: Operation		Road type :	2/2UD			Length (km) :	7.500								
		Time period :				Case number:									
FREE FLOW SPEEDS															
Option to enter other free flow speeds: No															
Di- rec- tion	Base free-flow speed FVo (km/h)					Carriage- way width adjust- ment, FVw Tab E2:1 (km/h)	FVo+FVw Light (2)+(3) (km/h)	Adjustment factors			Actual free-flow speeds, km/h				
	for different vehicles Table B-1:1 or B-1:2 LV MHV LB LT MC (2)							Side friction FFVsf Tab B3:1 (4)	Land use FFVrc Tab B4:1 (6)	Light (4*5*6) (7)	Other vehicle types	MHV	LB	LT	MC
1+2	68.0	60.0	73.0	58.0	55.0	0.0	68.0								
Comments:										User FFV, dir1: None! dir2:					
CAPACITY															
Direc- tion	Base Capacity		Adjustment factors for capacity					Actual capacity, C							
	Co Table C-1:1 pcu/h (11)		Carriageway width FCw Table C-2:1 (12)		Directional split FCsp Table C-3:1 (13)		Side friction FCsf Table C-4:1 (14)			C= Co*FCw*FCsp*FCsf pcu/h (11)*(12)*(13)*(14) (15)					
1+2	3100		1.000		0.970		0.970			2918					
ACTUAL SPEED and TRAVEL TIME for light vehicles										Only 2/2UD roads					
Di- rec- tion	Traffic flow, Q Form IR-2 pcu/h (21)	Degree of saturation DS=Q/C (21)/(15) (22)	Actual speed, Vlv Fig D2:1/:2 km/h (23)	Road segment length, L km (24)	Travel time, TT (24/23) sec (25)	ACTUAL SPEEDS				Di- rec- tion	Degree of bunching DB Fig D3:1 (31)				
						for other vehicle types km/h									
1+2	2828	0.969	31.41	7.500	859.426	31.09	31.61	31.01	30.89	1+2	0.903				
Space for user remark:															
Program version 1.10F Date of run: 140610/6:39															

KAJI -- INTERURBAN ROADS Province: JAWA TIMUR Date: JUNI 2014
 Link number: ARIF
 Form IR-2: Input Segment code: Checked by: ARIF
 TRAFFIC FLOW, SIDE FRICTION Administr. road class : NATIONAL Functional road class: ARTERIAL
 Road type : 2/2UD Length (km) : 7.500
 Purpose: Operation Time period : Case number:

TRAFFIC DATA:

Type of traffic data	ANNUAL AVERAGE DAILY TRAFFIC		DIRECTIONAL SPLIT
CLASSIFIED-HOURLY	AADT	K-factor	Dir1 - Dir2
(Class/Aadt/UNclass)	(veh/day)	(default: 0.11)	(default: 50 - 50)
			NA - NA %

Traffic Composition(%)	LV (%)	MHV (%)	LB (%)	LT (%)	MC (%)	Total (%)	LV = Light Vehicle
User values	31.67	34.16	0.134	3.597	30.43	100.0	MHV = Medium Heavy Vehicle
(normal values)	(57.0)	(23.0)	(7.0)	(4.0)	(9.0)	(100.0)	LB = Large Bus
							LT = Large Truck
							MC = MotorCycle

Traffic flow data for whole segment analysis:

Row	Dir	Light Vehicle	Med Heavy Veh	Large Bus	Large Truck	MotorCycle	Total flow Q	
1.1	Dir1	pce,1= 1.00	pce,1= 1.30	pce,1= 1.50	pce,1= 2.50	pce,1= 0.50		
1.2	Dir2	pce,2= 1.00	pce,2= 1.30	pce,2= 1.50	pce,2= 2.50	pce,2= 0.50		
		veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	Split veh/h pcu/h	
2	(1)	(2) (3)	(4) (5)	(6) (7)	(8) (9)	(10) (11)	(12) (13) (14)	
3	Dir1	484	484	624	811	2	3	
4	Dir2	458	458	392	510	2	3	
5	1+2	942	942	1016	1321	4	6	
6	Note.	If specific grade then				Directional split, SP= Q1/(Q1+Q2)=		54.9% 56.1%
7		dir 1 = uphill, dir 2= downhill				Pcu-factor, Fpcu =		1.005

SIDE FRICTION CLASS: If detailed data are available, use first table to determine weighted frequency of events and then go to second table. If not, use second table only.

1. Determination of frequency of events

Calculation of weighted frequency of events per hour and 200 m of the studied road segment.	Side friction type of events (20)	Symbol	Weighting factor (22)	Frequency of events (23)	Weighted frequency (24)
Frequencies are for both sides of the road.	Pedestrians	PED	0.6	NA / h,200m	NA
	Parking, stopping veh.	PSV	0.8	NA / h,200m	NA
	Entry+exit of vehicles	EEV	1.0	NA / h,200m	NA
	Slow-moving vehicles	SMV	0.4	NA / h	NA
Total:					NA

2. Determination of side friction class

Weighted frequency of events (30)	Typical conditions	Side friction class
< 50	Rural, agriculture or undeveloped with very few activities	VL= very low
50 - 149	Rural, some roadside buildings and some activities	L= low
150 - 249	Village, residential activities	M= medium
250 - 349	Village, some market activities	H= high
> 350	Almost urban, market and business activities	VH= very high
For current case indicate side friction class: L (L is default)		

KAJI -- INTERURBAN ROADS		Province:	JAWA TIMUR	Date:	JUNI 2014
Form IR-3: Analysis		Link number:		Handled by:	ARIF
		Segment code:		Checked by:	ARIF
SPEED, CAPACITY		Administr. road class :	NATIONAL	Functional road class:	ARTERIAL
Purpose: Operation		Road type :	2/2UD	Length (km)	7.500
		Time period :		Case number:	

FREE FLOW SPEEDS														
Option to enter other free flow speeds: No														
Di- rec- tion	Base free-flow speed FVo (km/h)					Carriage- way width adjust- ment, FVw Tab E2:1 (km/h)	FVo+FVw Light (2)+(3) (km/h)	Adjustment factors			Actual free-flow speeds, km/h			
	for different vehicles Table B-1:1 or B-1:2 LV MHV LB LT MC (2)							Side friction FFVsf Tab B3:1 (4)	Land use FFVrc Tab B4:1 (5)	Light types (4*5*6) (7)	Other vehicle types MHV LB LT MC			
1+2	68.0	60.0	73.0	58.0	55.0	0.0	68.0	0.970	1.000	65.96	58.20	70.81	56.26	53.35
Comments:										User FFV, dir1: None!			dir2:	

CAPACITY						
Direc- tion	Base Capacity		Adjustment factors for capacity			Actual capacity, C
	Co pcu/h Table C-1:1 (11)	Carriageway width FCw Table C-2:1 (12)	Directional split FCsp Table C-3:1 (13)	Side friction FCsf Table C-4:1 (14)	C= Co*FCw*FCsp*FCsf pcu/h (11)*(12)*(13)*(14) (15)	
1+2	3100	1.000	0.970	0.970	2918	

ACTUAL SPEED and TRAVEL TIME for light vehicles						Only 2/2UD roads					
Di- rec- tion	Traffic flow, Q Form IR-2 pcu/h (21)	Degree of saturation DS=Q/C (21)/(15) (22)	Actual speed, Vlv Fig D2:1/:2 km/h (23)	Road segment length, L km (24)	Travel time, TT (24/23) sec (25)	ACTUAL SPEEDS for other vehicle types km/h				Di- rec- tion	Degree of bunching DB Fig D3:1 (31)
						MHV	LB	LT	MC		
1+2	2990	1.025	NA	7.500	NA	NA	NA	NA	NA	1+2	0.916

Space for user remark:

Program version 1.10F | Date of run: 140610/6:40 |

KAJI -- INTERURBAN ROADS Province: JAWA TIMUR Date: JUNI 2014
 Link number: Handled by: ARIF
 Form IR-2: Input Segment code: Checked by: ARIF
 TRAFFIC FLOW, SIDE FRICTION Administr. road class : NATIONAL Functional road class: ARTERIAL
 Road type : 2/2UD Length (km) : 7.500
 Purpose: Operation Time period : Case number:

TRAFFIC DATA

Type of traffic data	ANNUAL AVERAGE DAILY TRAFFIC		DIRECTIONAL SPLIT
CLASSIFIED-HOURLY	AADT	K-factor	Dir1 - Dir2
(Class/Aadt/UNclass)	(veh/day)	(default: 0.11)	(default: 50 - 50)
			NA - NA %

Traffic Composition(%)	LV (%)	MHV (%)	LB (%)	LT (%)	MC (%)	Total (%)	LV = Light Vehicle
User values	31.71	34.10	0.191	3.605	30.37	100.0	MHV = Medium Heavy Vehicle
(normal values)	(57.0)	(23.0)	(7.0)	(4.0)	(9.0)	(100.0)	LB = Large Bus
							LT = Large Truck
							MC = MotorCycle

Traffic flow data for whole segment analysis:

Row	Di-	Light Vehicle	Med Heavy Veh	Large Bus	Large Truck	MotorCycle	Total flow Q	
1.1	tion	pce,1= 1.00	pce,1= 1.30	pce,1= 1.50	pce,1= 2.50	pce,1= 0.50		
1.2		pce,2= 1.00	pce,2= 1.30	pce,2= 1.50	pce,2= 2.50	pce,2= 0.50		
		veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	Split veh/h pcu/h	
2	(1)	(2) (3)	(4) (5)	(6) (7)	(8) (9)	(10) (11)	(12) (13) (14)	
3	Dir1	511 511	656 853	3 5	63 158	489 245	54.94 1722 1772	
4	Dir2	483 483	413 537	3 5	50 125	463 232	45.05 1412 1382	
5	1+2	994 994	1069 1390	6 10	113 283	952 477	3134 3154	
6	Note.	If specific grade then				Directional split. SP= Q1/(Q1+Q2)=		54.9% 56.1%
7		dir 1 = uphill, dir 2 = downhill				Pcu-factor, Fpcu =		1.006

SIDE FRICTION CLASS: If detailed data are available, use first table to determine weighted frequency of events and then go to second table. If not, use second table only.

1. Determination of frequency of events

Calculation of weighted frequency of events per hour and 200 m of the studied road segment.	Side friction type of events (20)	Symbol	Weighting factor (22)	Frequency of events (23)	Weighted frequency (24)
Frequencies are for both sides of the road.	Pedestrians	PED	0.6	NA / h,200m	NA
	Parking, stopping veh.	PSV	0.8	NA / h,200m	NA
	Entry+exit of vehicles	EEV	1.0	NA / h,200m	NA
	Slow-moving vehicles	SMV	0.4	NA / h	NA
Total:					NA

2. Determination of side friction class

Weighted frequency of events (30)	Typical conditions	Side friction class
< 50	Rural, agriculture or undeveloped with very few activities	VL= very low
50 - 149	Rural, some roadside buildings and some activities	L= low
150 - 249	Village, residential activities	M= medium
250 - 349	Village, some market activities	H= high
> 350	Almost urban, market and business activities	VH= very high
For current case indicate side friction class: L (L is default)		

KAJI -- INTERURBAN ROADS		Province:	JAWA TIMUR			Date:	JUNI 2014								
Form IR-3: Analysis		Link number:				Handled by:	ARIF								
		Segment code:				Checked by:	ARIF								
SPEED, CAPACITY		Administr. road class :	NATIONAL			Functional road class:	ARTERIAL								
Purpose: Operation		Road type :	2/2UD			Length (km)	7.500								
		Time period :				Case number:									
FREE FLOW SPEEDS															
Option to enter other free flow speeds: No															
Di- rec- tion	Base free-flow speed FVo (km/h)					Carriage-way width adjust-ment, FVw		FVo+FVw Light	Adjustment factors		Actual free-flow speeds, km/h				
	Table B-1:1 or B-1:2					Tab B2:1		(2)+(3)	Side friction FVsf	Land use	FFVlv = (FVo+FVw)*FFVsf*FFVrc	Other vehicle types			
	LV	MHV	LB	LT	MC	(km/h)	(km/h)	Tab B3:1	FFVrc	Tab B4:1	(4)*(5)*(6)	MHV	LB	LT	MC
	(2)					(3)	(4)	(5)	(6)	(7)					
1+2	68.0	60.0	73.0	58.0	55.0	0.0	68.0	0.970	1.000	65.96	58.20	70.81	56.26	53.35	
Comments:										User FFV, dir1: None! dir2:					
CAPACITY															
Direc- tion	Base Capacity		Adjustment factors for capacity					Actual capacity, C							
	Co		Carriageway width		Directional split		Side friction		C= Co*FCw*FCsp*FCsf pcu/h						
	Table C-1:1		Table C-2:1		Table C-3:1		Table C-4:1		(11)*(12)*(13)*(14)						
	pcu/h														
	(11)		(12)		(13)		(14)						(15)		
1+2	3100		1.000		0.970		0.970						2918		
ACTUAL SPEED and TRAVEL TIME for light vehicles										Only 2/2UD roads					
Di- rec- tion	Traffic flow, Q	Degree of saturation	Actual speed, Vlv	Road segment length, L	Travel time, TT	ACTUAL SPEEDS				Di- rec- tion	Degree of bunching DB				
	Form IR-2	DS=Q/C	Fig D2:1/:2	km	(24/23)	for other vehicle types									
	pcu/h	(21)/(15)	Km/h		sec	km/h							Fig D3:1		
	(21)	(22)	(23)	(24)	(25)	MHV	LB	LT	MC		(31)				
1+2	3154	1.081	NA	7.500	NA	NA	NA	NA	NA	1+2	0.928				
Space for user remark:															
Program version 1.10F Date of run: 140610/6:42															

KAJI -- INTERURBAN ROADS Province: JAWA TIMUR Date: JUNI 2014
 Link number: Handled by: ARIF
 Form IR-2: Input Segment code: Checked by: ARIF
 TRAFFIC FLOW, SIDE FRICTION Administr. road class : NATIONAL Functional road class: ARTERIAL
 Road type : 4/2UD Length (km) : 7.500
 Purpose: Operation Time period : Case number:

TRAFFIC DATA:

Type of traffic data	ANNUAL AVERAGE DAILY TRAFFIC		DIRECTIONAL SPLIT
CLASSIFIED-HOURLY	AADT	K-factor	Dir1 - Dir2
(Class/Aadt/UNclass)	(veh/day)	(default: 0.11)	(default: 50 - 50)
			NA - NA %

Traffic Composition(%)	LV (%)	MHV (%)	LB (%)	LT (%)	MC (%)	Total (%)	LV = Light Vehicle
User values	31.05	34.42	0.197	3.412	30.90	100.0	MHV = Medium Heavy Vehicle
(normal values)	(57.0)	(23.0)	(7.0)	(4.0)	(9.0)	(100.0)	LB = Large Bus
							LT = Large Truck
							MC = MotorCycle

Traffic flow data for whole segment analysis:

Row	Dir	Light Vehicle	Med Heavy Veh	Large Bus	Large Truck	MotorCycle	Total flow Q	
1.1	Dir1	pce,1= 1.00	pce,1= 1.44	pce,1= 1.46	pce,1= 2.10	pce,1= 0.64		
1.2	Dir2	pce,2= 1.00	pce,2= 1.44	pce,2= 1.46	pce,2= 2.10	pce,2= 0.64		
		veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	Split veh/h pcu/h	
2	(1)	(2) (3)	(4) (5)	(6) (7)	(8) (9)	(10) (11)	(%) (12) (13) (14)	
3	Dir1	322	322	428	617	2	3 38 80 321 206 54.94 1111 1228	
4	Dir2	306	306	268	386	2	3 31 65 304 195 45.05 911 955	
5	1+2	628	628	696	1003	4	6 69 145 625 401 2022 2183	
6	Note.	If specific grade then				Directional split. SP= Q1/(Q1+Q2)=		54.9% 56.2%
7		dir 1 = uphill, dir 2= downhill				Pcu-factor, Fpcu =		1.079

SIDE FRICTION CLASS: If detailed data are available, use first table to determine weighted frequency of events and then go to second table. If not, use second table only.

1. Determination of frequency of events

Calculation of weighted frequency of events per hour and 200 m of the studied road segment.	Side friction type of events	Symbol	Weighting factor	Frequency of events	Weighted frequency
	(20)	(21)	(22)	(23)	(24)
Frequencies are for both sides of the road.	Pedestrians	PED	0.6	NA / h,200m	NA
	Parking, stopping veh.	PSV	0.8	NA / h,200m	NA
	Entry+exit of vehicles	EEV	1.0	NA / h,200m	NA
	Slow-moving vehicles	SMV	0.4	NA / h	NA
				Total:	NA

2. Determination of side friction class

Weighted frequency of events (30)	Typical conditions	Side friction class
< 50	Rural, agriculture or undeveloped with very few activities	VL= very low
50 - 149	Rural, some roadside buildings and some activities	L= low
150 - 249	Village, residential activities	M= medium
250 - 349	Village, some market activities	H= high
> 350	Almost urban, market and business activities	VH= very high
For current case indicate side friction class: L (L is default)		

KAJI -- INTERURBAN ROADS		Province:	JAWA TIMUR			Date:	JUNI 2014							
Form IR-3: Analysis		Link number:				Handled by:	ARIF							
		Segment code:				Checked by:	ARIF							
SPEED, CAPACITY		Administr. road class :	NATIONAL			Functional road class:	ARTERIAL							
Purpose: Operation		Road type :	4/2UD			Length (km)	7.500							
		Time period :				Case number:								
FREE FLOW SPEEDS														
Option to enter other free flow speeds: No														
Di- rec- tion	Base free-flow speed FVo (km/h)					Carriage-way width adjust-ment, FVw		FVo+FVw Light	Adjustment factors		Actual free-flow speeds, km/h			
	Table B-1:1 or B-1:2					Tab B2:1		(2)+(3)	Side friction	Land use	FFVlv = (FVo+FVw)*FFVsf*FFVrc			
	LV	MHV	LB	LT	MC	(km/h)	(km/h)	Tab B3:1	FFVrc	Light	Other vehicle types			
(2)	(2)	(2)	(2)	(2)	(3)	(4)	(5)	(6)	(7)	MHV	LB	LT	MC	
1+2	74.0	63.0	78.0	60.0	60.0	-3.0	71.0	0.970	1.000	68.87	58.63	72.59	55.84	55.84
Comments:										User FFV, dir1: None dir2:				
CAPACITY														
Direc- tion	Base Capacity		Adjustment factors for capacity					Actual capacity, C						
	Co		Carriageway width		Directional split		Side friction		C= Co*FCw*FCsp*FCsf pcu/h					
	Table C-1:1		Table C-2:1		Table C-3:1		Table C-4:1		(11)*(12)*(13)*(14)					
	pcu/h													
	(11)		(12)		(13)		(14)							(15)
1+2	6800		0.910		0.975		0.970							5854
ACTUAL SPEED and TRAVEL TIME for light vehicles										Only 2/2UD roads				
Di- rec- tion	Traffic flow, Q	Degree of saturation	Actual speed, Vlv	Road segment length, L	Travel time, TT	ACTUAL SPEEDS				Di- rec- tion	Degree of bunching DB			
	Form IR-2	DS=Q/C	Fig D2:1/:2	km	sec	for other vehicle types								
	pcu/h	(21)/(15)	Km/h	km	sec	km/h								
	(21)	(22)	(23)	(24)	(25)	MHV	LB	LT	MC		(31)			
1+2	2183	0.373	62.74	7.500	430.309	53.41	66.13	50.87	50.87					
Space for user remark:														
Program version 1.10F										Date of run: 141015/9:14				

K A J I	Province	JAWA TIMUR	Date	JUNI 2014		
INTERURBAN ROADS	Link number:		Handled by :	ARIF		
	Segment code:		Checked by :	ARIF		
Form IR-1: Input	Segment between	BULU and	TUBAN			
	Specific grade:	No [NO indicates segment, YES spec grade(only 2/2UD)]				
GENERAL DATA,	Administr. road class :	NATIONAL	Functional road class:	ARTERIAL		
ROAD GEOMETRY	Road type	4/2UD	Length (km)	7.500		
Purpose: Operation	Time period:		Case number:			
HORIZONTAL ALIGNMENT						
To:	<pre> +---+ A * * * * * +---+ To: <----- * * * * * BULU * * * * * * * * * * N Indicate * * * * * * * * * * +---+ B * * * * * * * * * * </pre>			TUBAN		
	Horizontal curvature (radians/km):	NA	Roadside development	Side A Side B Mean		
	Sight distance > 300 m (%):	70	Default: 0%	0% 0% 0%		
	Sight distance class (default= B):	A				
VERTICAL ALIGNMENT						
	Only for specific grade analysis					
	Rise+fall :	NA m/km	Grade length (km) :			
	Alignment type:	FLAT (FLAT = default)	Grade alope (%):			
			Climbing lane (Y/N):			
CROSS SECTION						
Undivided road	side A	WsA	WcA	WcB	WsB	side B
		1.50	5.75	5.75	1.50	
	UNADJUSTED WIDTHS	Side A	Side B	Total	Mean	
	Average carriageway width, Wc (m)	5.75	5.75	11.50		
	Unobstructed shoulder width, Ws (m)	1.50	1.50	3.00	1.50	
ROAD SURFACE CONDITIONS						
CARRIAGEWAY SURFACE CONDITIONS		Side A	Side B			
Type [Flexible(asphalt)/Concrete/Other]		FLEXIBLE	FLEXIBLE			
Surface condition [Good/Fair/Bad]		GOOD	GOOD			
SHOULDER SURFACE CONDITIONS						
		Outer	Inner	Inner	Outer	
Surface type [Flexible/Concrete/Other]		OTHER			OTHER	
Drop from carriageway to shoulder (cm)		10			10	
Usability [Traffic/Parking/Emergency] (default shoulder usability)		EMERGENCY	(PARKING)	()	EMERGENCY	(PARKING)
EFFECTIVE WIDTHS						
Undivided road	Widths (m)	Divided road		Side A	Side B	
Shoulder, total	3.00	Shoulder, total				
Shoulder, mean	1.50	Shoulder, mean				
Carriageway	11.50	Carriageway				
TRAFFIC CONTROL CONDITIONS						
Speed limit	NA km/h	Max gross weight:	0.000 tonnes			
Other limitations						
More remarks						
Program version 1.10F Date of run: 141015/9:21						

KAJI -- INTERURBAN ROADS Province: JAWA TIMUR Date: JUNI 2014
 Link number: Handled by: ARIF
 Form IR-2: Input Segment code: Checked by: ARIF
 TRAFFIC FLOW, SIDE FRICTION Administr. road class : NATIONAL Functional road class: ARTERIAL
 Road type : 4/2UD Length (km) : 7.500
 Purpose: Operation Time period : Case number:

TRAFFIC DATA

Type of traffic data	ANNUAL AVERAGE DAILY TRAFFIC		DIRECTIONAL SPLIT
CLASSIFIED-HOURLY	ADDT	K-factor	Dir1 - Dir2
(Class/Aadt/UNclass)	(veh/day)	(default: 0.11)	(default: 50 - 50)
			NA - NA %

Traffic Composition(%)	LV (%)	MHV (%)	LB (%)	LT (%)	MC (%)	Total (%)	LV = Light Vehicle
User values	31.17	34.34	0.183	3.484	30.81	100.0	MHV = Medium Heavy Vehicle
(normal values)	(57.0)	(23.0)	(7.0)	(4.0)	(9.0)	(100.0)	LB = Large Bus
							LT = Large Truck
							MC = MotorCycle

Traffic flow data for whole segment analysis:

Row	Dir	Light Vehicle	Med Heavy Veh	Large Bus	Large Truck	MotorCycle	Total flow Q	
1.1	Dir1	pce,1= 1.00	pce,1= 1.46	pce,1= 1.49	pce,1= 2.16	pce,1= 0.66		
1.2	Dir2	pce,2= 1.00	pce,2= 1.46	pce,2= 1.49	pce,2= 2.16	pce,2= 0.66		
		veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	Split veh/h pcu/h	
2	(1)	(2) (3)	(4) (5)	(6) (7)	(8) (9)	(10) (11)	(%) (12) (13) (14)	
3	Dir1	349	349	460	673	2	3	
4	Dir2	331	331	289	423	2	3	
5	1+2	680	680	749	1096	4	6	
6	Note.	If specific grade then				Directional split, SP= Q1/(Q1+Q2)=		54.9% 56.2%
7		dir 1 = uphill, dir 2= downhill				Pcu-factor, Fpcu =		1.095

SIDE FRICTION CLASS: If detailed data are available, use first table to determine weighted frequency of events and then go to second table. If not, use second table only.

1. Determination of frequency of events

Calculation of weighted frequency of events per hour and 200 m of the studied road segment.	Side friction type of events (20)	Symbol	Weighting factor (22)	Frequency of events (23)	Weighted frequency (24)
Frequencies are for both sides of the road.	Pedestrians	PED	0.6	NA / h,200m	NA
	Parking, stopping veh.	PSV	0.8	NA / h,200m	NA
	Entry+exit of vehicles	EEV	1.0	NA / h,200m	NA
	Slow-moving vehicles	SMV	0.4	NA / h	NA
Total:					NA

2. Determination of side friction class

Weighted frequency of events (30)	Typical conditions	Side friction class
< 50	Rural, agriculture or undeveloped with very few activities	VL= very low
50 - 149	Rural, some roadside buildings and some activities	L= low
150 - 249	Village, residential activities	M= medium
250 - 349	Village, some market activities	H= high
> 350	Almost urban, market and business activities	VH= very high
For current case indicate side friction class: L (L is default)		

KAJI -- INTERURBAN ROADS	Province: JAWA TIMUR	Date: JUNI 2014
Form IR-3: Analysis	Link number:	Handled by: ARIF
	Segment code:	Checked by: ARIF
SPEED, CAPACITY	Administr. road class : NATIONAL	Functional road class: ARTERIAL
	Road type : 4/2UD	Length (km) : 7.500
Purpose: Operation	Time period :	Case number:

FREE FLOW SPEEDS
Option to enter other free flow speeds: No

Di- rec- tion	Base free-flow speed FVo (km/h)					Carriage- way width adjust- ment, FVw Tab E2:1 (3)	FVo+FVw Light (2)+(3) (4)	Adjustment factors			Actual free-flow speeds, km/h				
	for different vehicles Table B-1:1 or B-1:2 LV MHV LB LT MC (2)							Side friction FFVsf Tab B3:1 (5)	Land use FFVrc Tab B4:1 (6)	Light (4*5*6) (7)	Other vehicle types MHV LB LT MC				
1+2	74.0	63.0	78.0	60.0	60.0	-3.0	71.0	0.970	1.000	68.87	58.63	72.59	55.84	55.84	

Comments: User FFV, dir1: None; dir2:

Direc- tion	Base Capacity	Adjustment factors for capacity				Actual capacity, C
	Co Table C-1:1 pcu/h (11)	Carriageway width FCw Table C-2:1 (12)	Directional split FCsp Table C-3:1 (13)	Side friction FCsf Table C-4:1 (14)	C= Co*FCw*FCsp*FCsf (11)*(12)*(13)*(14) (15)	
1+2	6800	0.910	0.975	0.970	5854	

ACTUAL SPEED and TRAVEL TIME for light vehicles										Only 2/2UD roads			
Di- rec- tion	Traffic flow, Q Form IR-2 pcu/h (21)	Degree of saturation DS=Q/C (21)/(15) (22)	Actual speed, Vlv Fig D2:1/:2 km/h (23)	Road segment length, L km (24)	Travel time, TT (24/23) sec (25)	ACTUAL SPEEDS for other vehicle types km/h				Di- rec- tion	Degree of bunching DB Fig D3:1 (31)		
						MHV	LB	LT	MC				
1+2	2390	0.408	61.97	7.500	435.675	52.76	65.32	50.24	50.24				

Space for user remark:

Program version 1.10F | Date of run: 141015/9:21 |

K A J I		Province	JAWA TIMUR	Date	JUNI 2014																		
INTERURBAN ROADS		Link number:		Handled by :	ARIF																		
		Segment code:		Checked by :	ARIF																		
Form IR-1: Input		Segment between	BULU and	TUBAN																			
		Specific grade:	No [NO indicates segment, YES spec grade(only 2/2UD)]																				
GENERAL DATA,		Administr. road class :	NATIONAL	Functional road class:	ARTERIAL																		
ROAD GEOMETRY		Road type	4/2UD	Length (km)	7.500																		
Purpose: Operation		Time period:		Case number:																			
HORIZONTAL ALIGNMENT																							
To:		<pre> +---> A * * * * * +---> To: <----- * * * * * BULU * * * * * * * * * * N Indicate * * * * * * * * * * +---> B * * * * * </pre>		TUBAN																			
<table border="1"> <tr> <td>Horizontal curvature (radians/km):</td> <td>NA</td> <td>Roadside development</td> <td>Side A</td> <td>Side B</td> <td>Mean</td> </tr> <tr> <td>Sight distance > 300 m (%):</td> <td>70</td> <td>Default: 0%</td> <td>0 %</td> <td>0 %</td> <td>0 %</td> </tr> <tr> <td>Sight distance class (default= B):</td> <td>A</td> <td></td> <td></td> <td></td> <td></td> </tr> </table>		Horizontal curvature (radians/km):	NA	Roadside development	Side A	Side B	Mean	Sight distance > 300 m (%):	70	Default: 0%	0 %	0 %	0 %	Sight distance class (default= B):	A								
Horizontal curvature (radians/km):	NA	Roadside development	Side A	Side B	Mean																		
Sight distance > 300 m (%):	70	Default: 0%	0 %	0 %	0 %																		
Sight distance class (default= B):	A																						
VERTICAL ALIGNMENT																							
<table border="1"> <tr> <td>Rise+fall :</td> <td>NA m/km</td> <td>Grade length (km) :</td> <td></td> </tr> <tr> <td>Alignment type:</td> <td>FLAT (FLAT = default)</td> <td>Grade slope (%):</td> <td></td> </tr> <tr> <td></td> <td></td> <td>Climbing lane (Y/N):</td> <td></td> </tr> </table>		Rise+fall :	NA m/km	Grade length (km) :		Alignment type:	FLAT (FLAT = default)	Grade slope (%):				Climbing lane (Y/N):		<table border="1"> <tr> <td colspan="4">Only for specific grade analysis</td> </tr> </table>				Only for specific grade analysis					
Rise+fall :	NA m/km	Grade length (km) :																					
Alignment type:	FLAT (FLAT = default)	Grade slope (%):																					
		Climbing lane (Y/N):																					
Only for specific grade analysis																							
CROSS SECTION																							
<table border="1"> <tr> <td>Undivided road</td> <td>side A</td> <td>Wsa</td> <td>Wca</td> <td>Wcb</td> <td>Wsb</td> <td>side B</td> </tr> <tr> <td></td> <td></td> <td>1.50</td> <td>5.75</td> <td>5.75</td> <td>1.50</td> <td></td> </tr> </table>						Undivided road	side A	Wsa	Wca	Wcb	Wsb	side B			1.50	5.75	5.75	1.50					
Undivided road	side A	Wsa	Wca	Wcb	Wsb	side B																	
		1.50	5.75	5.75	1.50																		
<table border="1"> <tr> <td>UNADJUSTED WIDTHS</td> <td>Side A</td> <td>Side B</td> <td>Total</td> <td>Mean</td> </tr> <tr> <td>Average carriageway width, Wc (m)</td> <td>5.75</td> <td>5.75</td> <td>11.50</td> <td></td> </tr> <tr> <td>Unobstructed shoulder width, Ws (m)</td> <td>1.50</td> <td>1.50</td> <td>3.00</td> <td>1.50</td> </tr> </table>						UNADJUSTED WIDTHS	Side A	Side B	Total	Mean	Average carriageway width, Wc (m)	5.75	5.75	11.50		Unobstructed shoulder width, Ws (m)	1.50	1.50	3.00	1.50			
UNADJUSTED WIDTHS	Side A	Side B	Total	Mean																			
Average carriageway width, Wc (m)	5.75	5.75	11.50																				
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ROAD SURFACE CONDITIONS																							
CARRIAGEWAY SURFACE CONDITIONS																							
Type [Flexible(asphalt)/Concrete/Other]		FLEXIBLE	FLEXIBLE																				
Surface condition [Good/Fair/Bad]		GOOD	GOOD																				
SHOULDER SURFACE CONDITIONS																							
		Outer	Inner	Inner	Outer																		
Surface type [Flexible/Concrete/Other]		OTHER			OTHER																		
Drop from carriageway to shoulder (cm)		10			10																		
Usability [Traffic/Parking/Emergency] (default shoulder usability)		EMERGENCY			EMERGENCY																		
		(PARKING)			(PARKING)																		
EFFECTIVE WIDTHS																							
Undivided road		Divided road		Widths (m)																			
Shoulder, total		Shoulder, total		Side A	Side B																		
Shoulder, mean		Shoulder, mean																					
Carriageway		Carriageway																					
TRAFFIC CONTROL CONDITIONS																							
Speed limit :		NA km/h	Max gross weight: 0.000 tonnes																				
Other limitations :																							
More remarks :																							
Program version 1.10F Date of run: 141015/9:22																							

KAJI -- INTERURBAN ROADS Province: JAWA TIMUR Date: JUNI 2014
 Link number: ARIF
 Form IR-2: Input Segment code: Checked by: ARIF
 TRAFFIC FLOW, SIDE FRICTION Administr. road class : NATIONAL Functional road class: ARTERIAL
 Road type : 4/2UD Length (km) : 7.500
 Purpose: Operation Time period : Case number:

TRAFFIC DATA:

Type of traffic data	ANNUAL AVERAGE DAILY TRAFFIC		DIRECTIONAL SPLIT	
CLASSIFIED-HOURLY	AADT	K-factor	Dir1 - Dir2	
(Class/Aadt/UNclass)	(veh/day)	(default: 0.11)	(default: 50 - 50)	
			NA - NA %	

Traffic Composition(%)	LV (%)	MHV (%)	LB (%)	LT (%)	MC (%)	Total (%)	LV = Light Vehicle
User values	31.33	34.28	0.171	3.463	30.73	100.0	MHV = Medium Heavy Vehicle
(normal values)	(57.0)	(23.0)	(7.0)	(4.0)	(9.0)	(100.0)	LB = Large Bus
							LT = Large Truck
							MC = MotorCycle

Traffic flow data for whole segment analysis:

Row	Dir	Light Vehicle	Med Heavy Veh	Large Bus	Large Truck	MotorCycle	Total flow Q	
1.1	Dir1	pce,1= 1.00	pce,1= 1.48	pce,1= 1.52	pce,1= 2.21	pce,1= 0.68		
1.2	Dir2	pce,2= 1.00	pce,2= 1.48	pce,2= 1.52	pce,2= 2.21	pce,2= 0.68		
		veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	Split veh/h pcu/h	
2	(1)	(2) (3)	(4) (5)	(6) (7)	(8) (9)	(10) (11)	(12) (13) (14)	
3	Dir1	376	376	493	731	2	3	
4	Dir2	357	357	309	458	2	3	
5	1+2	733	733	802	1189	4	6	
6	Note. If specific grade then					Directional split. SP= Q1/(Q1+Q2)=	54.9%	56.2%
7	dir 1 = uphill, dir 2= downhill					Pcu-factor, Fpcu =		1.110

SIDE FRICTION CLASS: If detailed data are available, use first table to determine weighted frequency of events and then go to second table. If not, use second table only.

1. Determination of frequency of events

Calculation of weighted frequency of events per hour and 200 m of the studied road segment.	Side friction type of events (20)	Symbol	Weighting factor (22)	Frequency of events (23)	Weighted frequency (24)
Frequencies are for both sides of the road.	Pedestrians	PED	0.6	NA / h,200m	NA
	Parking, stopping veh.	PSV	0.8	NA / h,200m	NA
	Entry+exit of vehicles	EEV	1.0	NA / h,200m	NA
	Slow-moving vehicles	SMV	0.4	NA / h	NA
Total:					NA

2. Determination of side friction class

Weighted frequency of events (30)	Typical conditions	Side friction class
< 50	Rural, agriculture or undeveloped with very few activities	VL= very low
50 - 149	Rural, some roadside buildings and some activities	L= low
150 - 249	Village, residential activities	M= medium
250 - 349	Village, some market activities	H= high
> 350	Almost urban, market and business activities	VH= very high
For current case indicate side friction class: L (L is default)		

KAJI -- INTERURBAN ROADS	Province: JAWA TIMUR	Date: JUNI 2014
Form IR-3: Analysis	Link number:	Handled by: ARIF
	Segment code:	Checked by: ARIF
SPEED, CAPACITY	Administr. road class : NATIONAL	Functional road class: ARTERIAL
Purpose: Operation	Road type : 4/2UD	Length (km) : 7.500
	Time period :	Case number:

FREE FLOW SPEEDS
Option to enter other free flow speeds: No

Di- rec- tion	Base free-flow speed FVo (km/h)					Carriage- way width adjust- ment, FVw Tab E2:1 (km/h)	FVo+FVw Light (2)+(3) (km/h)	Adjustment factors			Actual free-flow speeds, km/h				
	for different vehicles Table B-1:1 or B-1:2 LV MHV LB LT MC (2)							Side friction FFVsf Tab B3:1 (4)	Land use FFVrc Tab B4:1 (5)	Light FFVlf (6)	Other vehicle types (4*5*6) (7)	Light	Other vehicle	Light	Other vehicle
1+2	74.0	63.0	78.0	60.0	60.0	-3.0	71.0	0.970	1.000	68.87	58.63	72.59	55.84	55.84	

Comments: User FFV, dir1: None; dir2:

Direc- tion	Base Capacity		Adjustment factors for capacity				Actual capacity, C	
	Co pcu/h Table C-1:1 (11)	Carriageway width FCw Table C-2:1 (12)	Directional split FCsp Table C-3:1 (13)	Side friction FCsf Table C-4:1 (14)	C= Co*FCw*FCsp*FCsf (11)*(12)*(13)*(14) (15)	pcu/h	pcu/h	
1+2	6800	0.910	0.975	0.970	5854			

ACTUAL SPEED and TRAVEL TIME for light vehicles										Only 2/2UD roads			
Di- rec- tion	Traffic flow, Q Form IR-2 pcu/h (21)	Degree of saturation DS=Q/C (21)/(15) (22)	Actual speed, Vlv Fig D2:1/:2 km/h (23)	Road segment length, L km (24)	Travel time, TT (24/23) sec (25)	ACTUAL SPEEDS				Di- rec- tion	Degree of bunching DB Fig D3:1 (31)		
						for other vehicle types km/h							
1+2	2597	0.444	61.16	7.500	441.426	52.07	64.47	49.59	49.59				

Space for user remark:
Program version 1.10F | Date of run: 141015/9:22

KAJI -- INTERURBAN ROADS	Province: JAWA TIMUR	Date: JUNI 2014
Link number:	Handled by: ARIF	
Form IR-2: Input	Segment code:	Checked by: ARIF
TRAFFIC FLOW, SIDE FRICTION	Administr. road class : NATIONAL	Functional road class: ARTERIAL
Purpose: Operation	Road type : 4/2UD	Length (km) : 7.500
	Time period :	Case number:

TRAFFIC DATA:									
Type of traffic data		ANNUAL AVERAGE DAILY TRAFFIC					DIRECTIONAL SPLIT		
CLASSIFIED-HOURLY		AADT		K-factor			Dir1 - Dir2		
(Class/Aadt/UNclass)		(veh/day)		(default: 0.11)			(default: 50 - 50)		
							NA - NA %		
Traffic Composition(%)	LV (%)	MHV (%)	LB (%)	LT (%)	MC (%)	Total (%)	LV = Light Vehicle	MHV = Medium Heavy Vehicle	LB = Large Bus
User values	31.42	34.26	0.160	3.522	30.62	100.0	LT = Large Truck	MC = MotorCycle	
(normal values)	(57.0)	(23.0)	(7.0)	(4.0)	(9.0)	(100.0)			

Traffic flow data for whole segment analysis:

Row	Dir	Light Vehicle	Med Heavy Veh	Large Bus	Large Truck	MotorCycle	Total flow Q	
1.1	Dir1	pce,1= 1.00	pce,1= 1.50	pce,1= 1.55	pce,1= 2.26	pce,1= 0.70		
1.2	Dir2	pce,2= 1.00	pce,2= 1.50	pce,2= 1.55	pce,2= 2.26	pce,2= 0.70		
		veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	Split veh/h pcu/h	
2	(1)	(2) (3)	(4) (5)	(6) (7)	(8) (9)	(10) (11)	(12) (13) (14)	
3	Dir1	403	403	526	791	2	3	
4	Dir2	382	382	330	496	2	3	
5	1+2	785	785	856	1287	4	6	
6	Note.	If specific grade then				Directional split, SP= Q1/(Q1+Q2)=		54.9% 56.2%
7		dir 1 = uphill, dir 2= downhill				Pcu-factor, Fpcu =		1.126

SIDE FRICTION CLASS: If detailed data are available, use first table to determine weighted frequency of events and then go to second table. If not, use second table only.

1. Determination of frequency of events

Calculation of weighted frequency of events per hour and 200 m of the studied road segment.	Side friction type of events (20)	Symbol	Weighting factor (22)	Frequency of events (23)	Weighted frequency (24)
Frequencies are for both sides of the road.	Pedestrians	PED	0.6	NA / h,200m	NA
	Parking, stopping veh.	PSV	0.8	NA / h,200m	NA
	Entry+exit of vehicles	EEV	1.0	NA / h,200m	NA
	Slow-moving vehicles	SMV	0.4	NA / h	NA
Total:					NA

2. Determination of side friction class

Weighted frequency of events (30)	Typical conditions	Side friction class
< 50	Rural, agriculture or undeveloped with very few activities	VL= very low
50 - 149	Rural, some roadside buildings and some activities	L= low
150 - 249	Village, residential activities	M= medium
250 - 349	Village, some market activities	H= high
> 350	Almost urban, market and business activities	VH= very high
For current case indicate side friction class: L (L is default)		

KAJI -- INTERURBAN ROADS		Province:	JAWA TIMUR			Date:	JUNI 2014							
Form IR-3: Analysis		Link number:				Handled by:	ARIF							
		Segment code:				Checked by:	ARIF							
SPEED, CAPACITY		Administr. road class :	NATIONAL		Functional road class :	ARTERIAL								
Purpose: Operation		Road type :	4/2UD		Length (km) :	7.500								
		Time period :				Case number:								
FREE FLOW SPEEDS														
Option to enter other free flow speeds: No														
Di- rec- tion	Base free-flow speed FVo (km/h)					Carriage-way width adjust-ment, FVw		FVo+FVw Light	Adjustment factors		Actual free-flow speeds, km/h			
	Table B-1:1 or B-1:2					Tab B2:1		(2)+(3)	Side friction	Land use	FFVlv = (FVo+FVw)*FFVsf*FFVrc			
	LV	MHV	LB	LT	MC	(km/h)	(km/h)	Tab B3:1	Tab B4:1	Light Other vehicle types				
	(2)					(3)	(4)	(5)	(6)	(7)	MHV	LB	LT	MC
1+2	74.0	63.0	78.0	60.0	60.0	-3.0	71.0	0.970	1.000	68.87	58.63	72.59	55.84	55.84
Comments:										User FFV, dir1: None! dir2:				
CAPACITY														
Direc- tion	Base Capacity		Adjustment factors for capacity					Actual capacity, C						
	Co		Carriageway width		Directional split		Side friction		C= Co*FCw*FCsp*FCsf pcu/h					
	Table C-1:1		Table C-2:1		Table C-3:1		Table C-4:1		(11)*(12)*(13)*(14)					
	pcu/h													
	(11)		(12)		(13)		(14)							(15)
1+2	6800		0.910		0.975		0.970							5853
ACTUAL SPEED and TRAVEL TIME for light vehicles										Only 2/2UD roads				
Di- rec- tion	Traffic flow, Q	Degree of saturation	Actual speed, Vlv	Road segment length, L	Travel time, TT	ACTUAL SPEEDS				Di- rec- tion	Degree of bunching DB			
	Form IR-2	DS=Q/C	Fig D2:1/:2	km	sec	for other vehicle types								
	pcu/h	(21)/(15)	Km/h	km	sec	MHV	LB	LT	MC			Fig D3:1		
	(21)	(22)	(23)	(24)	(25)					(31)				
1+2	2815	0.481	60.27	7.500	447.939	51.31	63.53	48.87	48.87					
Space for user remark:														
Program version 1.10F										Date of run: 141015/9:22				

K A J I	Province	JAWA TIMUR	Date	JUNI 2014																									
INTERURBAN ROADS	Link number:		Handled by :	ARIF																									
	Segment code:		Checked by :	ARIF																									
Form IR-1: Input	Segment between	BULU and	TUBAN																										
	Specific grade:	No [NO indicates segment, YES spec grade(only 2/2UD)]																											
GENERAL DATA,	Administr. road class :	NATIONAL	Functional road class:	ARTERIAL																									
ROAD GEOMETRY	Road type :	4/2UD	Length (km) :	7.500																									
Purpose: Operation	Time period:		Case number:																										
HORIZONTAL ALIGNMENT																													
<pre> +---+ A * * * * * +---+ To: * * * * * To: <----- * * * * * BULU * * * * * * * * * * N Indicate * * * * * * * * * * +---+ B +--- north (N) </pre>																													
<table border="1"> <tr> <td>Horizontal curvature (radians/km):</td> <td>NA</td> <td>Roadside development</td> <td>Side A</td> <td>Side B</td> <td>Mean</td> </tr> <tr> <td>Sight distance > 300 m (%):</td> <td>70</td> <td>Default: 0%</td> <td>0%</td> <td>0%</td> <td>0%</td> </tr> <tr> <td>Sight distance class (default= B):</td> <td>A</td> <td></td> <td></td> <td></td> <td></td> </tr> </table>					Horizontal curvature (radians/km):	NA	Roadside development	Side A	Side B	Mean	Sight distance > 300 m (%):	70	Default: 0%	0%	0%	0%	Sight distance class (default= B):	A											
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SHOULDER SURFACE CONDITIONS	SIDE A		SIDE B																										
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Program version 1.10F Date of run: 141015/9:23																													

KAJI -- INTERURBAN ROADS Province: JAWA TIMUR Date: JUNI 2014
 Link number: Handled by: ARIF
 Form IR-2: Input Segment code: Checked by: ARIF
 TRAFFIC FLOW, SIDE FRICTION Administr. road class : NATIONAL Functional road class: ARTERIAL
 Road type : 4/2UD Length (km) : 7.500
 Purpose: Operation Time period : Case number:

TRAFFIC DATA:

Type of traffic data	ANNUAL AVERAGE DAILY TRAFFIC				DIRECTIONAL SPLIT	
CLASSIFIED-HOURLY	ADDT	K-factor		Dir1 - Dir2		
(Class/Aadt/UNclass)	(veh/day)	(default: 0.11)		(default: 50 - 50)		
				NA - NA %		

Traffic Composition(%)	LV (%)	MHV (%)	LB (%)	LT (%)	MC (%)	Total (%)	LV = Light Vehicle
User values	31.51	34.22	0.150	3.539	30.57	100.0	MHV = Medium Heavy Vehicle
(normal values)	(57.0)	(23.0)	(7.0)	(4.0)	(9.0)	(100.0)	LB = Large Bus
							LT = Large Truck
							MC = MotorCycle

Traffic flow data for whole segment analysis:

Row	Dir	Light Vehicle	Med Heavy Veh	Large Bus	Large Truck	MotorCycle	Total flow Q	
1.1	Dir1	pce,1= 1.00	pce,1= 1.52	pce,1= 1.59	pce,1= 2.31	pce,1= 0.72		
1.2	Dir2	pce,2= 1.00	pce,2= 1.52	pce,2= 1.59	pce,2= 2.31	pce,2= 0.72		
		veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	Split veh/h pcu/h	
2	(1)	(2) (3)	(4) (5)	(6) (7)	(8) (9)	(10) (11)	(%) (12) (13) (14)	
3	Dir1	430	430	558	850	2	3	
4	Dir2	407	407	351	535	2	3	
5	1+2	837	837	909	1385	4	6	
6	Note.	If specific grade then				Directional split. SP= Q1/(Q1+Q2)=		54.9% 56.2%
7		dir 1 = uphill, dir 2= downhill				Pcu-factor, Fpcu =		1.141

SIDE FRICTION CLASS: If detailed data are available, use first table to determine weighted frequency of events and then go to second table. If not, use second table only.

1. Determination of frequency of events

Calculation of weighted frequency of events per hour and 200 m of the studied road segment.	Side friction type of events (20)	Symbol	Weighting factor (22)	Frequency of events (23)	Weighted frequency (24)
Frequencies are for both sides of the road.	Pedestrians	PED	0.6	NA / h,200m	NA
	Parking, stopping veh.	PSV	0.8	NA / h,200m	NA
	Entry+exit of vehicles	EEV	1.0	NA / h,200m	NA
	Slow-moving vehicles	SMV	0.4	NA / h	NA
Total:					NA

2. Determination of side friction class

Weighted frequency of events (30)	Typical conditions	Side friction class
< 50	Rural, agriculture or undeveloped with very few activities	VL= very low
50 - 149	Rural, some roadside buildings and some activities	L= low
150 - 249	Village, residential activities	M= medium
250 - 349	Village, some market activities	H= high
> 350	Almost urban, market and business activities	VH= very high
For current case indicate side friction class: L (L is default)		

KAJI -- INTERURBAN ROADS		Province:	JAWA TIMUR			Date:	JUNI 2014							
Form IR-3: Analysis		Link number:				Handled by:	ARIF							
		Segment code:				Checked by:	ARIF							
SPEED, CAPACITY		Administr. road class :	NATIONAL		Functional road class :	ARTERIAL								
Purpose: Operation		Road type :	4/2UD		Length (km) :	7.500								
		Time period :				Case number:								
FREE FLOW SPEEDS														
Option to enter other free flow speeds: No														
Di- rec- tion	Base free-flow speed FVo (km/h)					Carriage-way width adjust-ment, FVw		FVo+FVw Light	Adjustment factors		Actual free-flow speeds, km/h			
	Table B-1:1 or B-1:2					Tab B2:1		(2)+(3)	Side friction	Land use	FFVlv = (FVo+FVw)*FFVsf*FFVrc			
	LV MHV LB LT MC					(km/h)		(km/h)	Tab B3:1	Tab B4:1	Light Other vehicle types			
	(2)					(3)		(4)	(5)	(6)	(7)	MHV LB LT MC		
1+2	74.0	63.0	78.0	60.0	60.0	-3.0	71.0	0.970	1.000	68.87	58.63	72.59	55.84	55.84
Comments:										User FFV, dir1: None! dir2:				
CAPACITY														
Direc- tion	Base Capacity		Adjustment factors for capacity					Actual capacity, C						
	Co		Carriageway width		Directional split		Side friction		C= Co*FCw*FCsp*FCsf pcu/h					
	Table C-1:1 pcu/h (11)		Table C-2:1 FCw (12)		Table C-3:1 FCsp (13)		Table C-4:1 FCsf (14)		(11)*(12)*(13)*(14) (15)					
1+2	6800		0.910		0.975		0.970		5854					
ACTUAL SPEED and TRAVEL TIME for light vehicles										Only 2/2UD roads				
Di- rec- tion	Traffic flow, Q	Degree of saturation	Actual speed, Vlv	Road segment length, L	Travel time, TT	ACTUAL SPEEDS				Di- rec- tion	Degree of bunching DB			
	Form IR-2 pcu/h (21)	DS=Q/C (21)/(15) (22)	Fig D2:1/:2 km/h (23)	km (24)	sec (25)	for other vehicle types km/h						Fig D3:1 (31)		
						MHV	LB	LT	MC					
1+2	3033	0.518	59.34	7.500	454.946	50.52	62.55	48.11	48.11					
Space for user remark:														
Program version 1.10F										Date of run: 141015/9:23				

KAJI -- INTERURBAN ROADS	Province:	JAWA TIMUR	Date:	JUNI 2014
Form IR-2: Input	Link number:		Handled by:	ARIF
	Segment code:		Checked by:	ARIF
TRAFFIC FLOW, SIDE FRICTION	Administr. road class :	NATIONAL	Functional road class:	ARTERIAL
	Road type :	4/2UD	Length (km) :	7.500
Purpose: Operation	Time period :		Case number:	

TRAFFIC DATA:									
Type of traffic data		ANNUAL AVERAGE DAILY TRAFFIC					DIRECTIONAL SPLIT		
CLASSIFIED-HOURLY		AADT		K-factor			Dir1 - Dir2		
(Class/Aadt/UNclass)		(veh/day)		(default: 0.11)			(default: 50 - 50)		
							NA - NA %		
Traffic Composition(%)	LV (%)	MHV (%)	LB (%)	LT (%)	MC (%)	Total (%)	LV = Light Vehicle	MHV = Medium Heavy Vehicle	LB = Large Bus
User values	31.58	34.17	0.142	3.587	30.51	100.0	LT = Large Truck	MC = MotorCycle	
(normal values)	(57.0)	(23.0)	(7.0)	(4.0)	(9.0)	(100.0)			

Traffic flow data for whole segment analysis:

Row	Dir	Light Vehicle	Med Heavy Veh	Large Bus	Large Truck	MotorCycle	Total flow Q							
1.1	Dir1	pce,1= 1.00	pce,1= 1.54	pce,1= 1.62	pce,1= 2.36	pce,1= 0.74								
1.2	Dir2	pce,2= 1.00	pce,2= 1.54	pce,2= 1.62	pce,2= 2.36	pce,2= 0.74								
		veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	Split (%)	veh/h pcu/h						
2	(1)	(2) (3)	(4) (5)	(6) (7)	(8) (9)	(10) (11)	(12)	(13) (14)						
3	Dir1	457	457	591	912	2	3	56	132	441	328	54.95	1547	1832
4	Dir2	432	432	371	573	2	3	45	106	418	311	45.04	1268	1425
5	1+2	889	889	962	1485	4	6	101	238	859	639		2815	3257
6	Note.	If specific grade then				Directional split, SP= Q1/(Q1+Q2)=		54.9%	56.2%					
7		dir 1 = uphill, dir 2= downhill				Pcu-factor, Fpcu =			1.157					

SIDE FRICTION CLASS: If detailed data are available, use first table to determine weighted frequency of events and then go to second table. If not, use second table only.

1. Determination of frequency of events

Calculation of weighted frequency of events per hour and 200 m of the studied road segment.	Side friction type of events (20)	Symbol	Weighting factor (22)	Frequency of events (23)	Weighted frequency (24)
Frequencies are for both sides of the road.	Pedestrians	PED	0.6	NA / h,200m	NA
	Parking, stopping veh.	PSV	0.8	NA / h,200m	NA
	Entry+exit of vehicles	EEV	1.0	NA / h,200m	NA
	Slow-moving vehicles	SMV	0.4	NA / h	NA
Total:					NA

2. Determination of side friction class

Weighted frequency of events (30)	Typical conditions	Side friction class
< 50	Rural, agriculture or undeveloped with very few activities	VL= very low
50 - 149	Rural, some roadside buildings and some activities	L= low
150 - 249	Village, residential activities	M= medium
250 - 349	Village, some market activities	H= high
> 350	Almost urban, market and business activities	VH= very high
For current case indicate side friction class: L (L is default)		

KAJI -- INTERURBAN ROADS	Province: JAWA TIMUR	Date: JUNI 2014
Form IR-3: Analysis	Link number:	Handled by: ARIF
	Segment code:	Checked by: ARIF
SPEED, CAPACITY	Administr. road class : NATIONAL	Functional road class: ARTERIAL
Purpose: Operation	Road type : 4/2UD	Length (km) : 7.500
	Time period :	Case number:

FREE FLOW SPEEDS
Option to enter other free flow speeds: No

Di- rec- tion	Base free-flow speed FVo (km/h)					Carriage- way width adjust- ment, FVw Tab E2:1 (km/h)	FVo+FVw Light (2)+(3) (km/h)	Adjustment factors			Actual free-flow speeds, km/h				
	for different vehicles Table B-1:1 or B-1:2 LV MHV LB LT MC (2)							Side friction FFVsf Tab B3:1 (4)	Land use FFVrc Tab B4:1 (5)	Light (4*5*6) (6)	Other vehicle types MHV LB LT MC (7)				
1+2	74.0	63.0	78.0	60.0	60.0	-3.0	71.0	0.970	1.000	68.87	58.63	72.59	55.84	55.84	

Comments: User FFV, dir1: None!
dir2:

Direc- tion	Base Capacity		Adjustment factors for capacity				Actual capacity, C	
	Co pcu/h Table C-1:1 (11)	Carriageway width FCw Table C-2:1 (12)	Directional split FCsp Table C-3:1 (13)	Side friction FCsf Table C-4:1 (14)	C= Co*FCw*FCsp*FCsf (11)*(12)*(13)*(14) (15)	pcu/h		
1+2	6800	0.910	0.975	0.970	5854			

ACTUAL SPEED and TRAVEL TIME for light vehicles										Only 2/2UD roads		
Di- rec- tion	Traffic flow, Q Form IR-2 pcu/h (21)	Degree of saturation DS=Q/C (21)/(15) (22)	Actual speed, Vlv Fig D2:1/:2 km/h (23)	Road segment length, L km (24)	Travel time, TT (24/23) sec (25)	ACTUAL SPEEDS				Di- rec- tion	Degree of bunching DB Fig D3:1 (31)	
						for other vehicle types km/h MHV LB LT MC (7)						
1+2	3257	0.556	58.34	7.500	462.773	49.67	61.49	47.30	47.30			

Space for user remark:

Program version 1.10F | Date of run: 141015/9:23 |

K A J I	Province	JAWA TIMUR	Date	JUNI 2014																														
INTERURBAN ROADS	Link number:		Handled by :	ARIF																														
	Segment code:		Checked by :	ARIF																														
Form IR-1: Input	Segment between	BULU and	TUBAN																															
	Specific grade:	No [NO indicates segment, YES spec grade(only 2/2UD)]																																
GENERAL DATA,	Administr. road class :	NATIONAL	Functional road class:	ARTERIAL																														
ROAD GEOMETRY	Road type :	4/2UD	Length (km) :	7.500																														
Purpose: Operation	Time period:		Case number:																															
HORIZONTAL ALIGNMENT																																		
<pre> +---+ A * * * * * +---+ To: * * * * * To: <----- * * * * * BULU * * * * * * * * * * N Indicate * * * * * * * * * * +---+ B +--- north (N) </pre>																																		
<table border="1"> <tr> <td>Horizontal curvature (radians/km):</td> <td>NA</td> <td>Roadside development</td> <td>Side A</td> <td>Side B</td> <td>Mean</td> </tr> <tr> <td>Sight distance > 300 m (%):</td> <td>70</td> <td>Default: 0%</td> <td>0%</td> <td>0%</td> <td>0%</td> </tr> <tr> <td>Sight distance class (default= B):</td> <td>A</td> <td></td> <td></td> <td></td> <td></td> </tr> </table>					Horizontal curvature (radians/km):	NA	Roadside development	Side A	Side B	Mean	Sight distance > 300 m (%):	70	Default: 0%	0%	0%	0%	Sight distance class (default= B):	A																
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<pre> Undivided road side A ##### side B WsA WcA WcB WsB 1.50 5.75 5.75 1.50 </pre> <table border="1"> <tr> <th>UNADJUSTED WIDTHS</th> <th>Side A</th> <th>Side B</th> <th>Total</th> <th>Mean</th> </tr> <tr> <td>Average carriageway width, Wc (m)</td> <td>5.75</td> <td>5.75</td> <td>11.50</td> <td></td> </tr> <tr> <td>Unobstructed shoulder width, Ws (m)</td> <td>1.50</td> <td>1.50</td> <td>3.00</td> <td>1.50</td> </tr> </table>					UNADJUSTED WIDTHS	Side A	Side B	Total	Mean	Average carriageway width, Wc (m)	5.75	5.75	11.50		Unobstructed shoulder width, Ws (m)	1.50	1.50	3.00	1.50															
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<table border="1"> <tr> <th>SHOULDER SURFACE CONDITIONS</th> <th colspan="2">SIDE A</th> <th colspan="2">SIDE B</th> </tr> <tr> <td></td> <td>Outer</td> <td>Inner</td> <td>Inner</td> <td>Outer</td> </tr> <tr> <td>Surface type [Flexible/Concrete/Other]</td> <td colspan="2">OTHER</td> <td colspan="2">OTHER</td> </tr> <tr> <td>Drop from carriageway to shoulder (cm)</td> <td colspan="2">10</td> <td colspan="2">10</td> </tr> <tr> <td>Usability [Traffic/Parking/Emergency]</td> <td colspan="2">EMERGENCY</td> <td colspan="2">EMERGENCY</td> </tr> <tr> <td>(default shoulder usability)</td> <td>(PARKING)</td> <td>()</td> <td>()</td> <td>(PARKING)</td> </tr> </table>					SHOULDER SURFACE CONDITIONS	SIDE A		SIDE B			Outer	Inner	Inner	Outer	Surface type [Flexible/Concrete/Other]	OTHER		OTHER		Drop from carriageway to shoulder (cm)	10		10		Usability [Traffic/Parking/Emergency]	EMERGENCY		EMERGENCY		(default shoulder usability)	(PARKING)	()	()	(PARKING)
SHOULDER SURFACE CONDITIONS	SIDE A		SIDE B																															
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(default shoulder usability)	(PARKING)	()	()	(PARKING)																														
EFFECTIVE WIDTHS																																		
<table border="1"> <tr> <th>Undivided road</th> <th>Widths (m)</th> <th>Divided road</th> <th colspan="2">Widths (m)</th> </tr> <tr> <td></td> <td></td> <td></td> <th>Side A</th> <th>Side B</th> </tr> <tr> <td>Shoulder, total</td> <td>3.00</td> <td>Shoulder, total</td> <td></td> <td></td> </tr> <tr> <td>Shoulder, mean</td> <td>1.50</td> <td>Shoulder, mean</td> <td></td> <td></td> </tr> <tr> <td>Carriageway</td> <td>11.50</td> <td>Carriageway</td> <td></td> <td></td> </tr> </table>					Undivided road	Widths (m)	Divided road	Widths (m)					Side A	Side B	Shoulder, total	3.00	Shoulder, total			Shoulder, mean	1.50	Shoulder, mean			Carriageway	11.50	Carriageway							
Undivided road	Widths (m)	Divided road	Widths (m)																															
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Program version 1.10F Date of run: 141015/9:23																																		

KAJI -- INTERURBAN ROADS	Province: JAWA TIMUR	Date: JUNI 2014
Form IR-2: Input	Link number:	Handled by: ARIF
	Segment code:	Checked by: ARIF
TRAFFIC FLOW, SIDE FRICTION	Administr. road class : NATIONAL	Functional road class: ARTERIAL
	Road type : 4/2UD	Length (km) : 7.500
Purpose: Operation	Time period :	Case number:

TRAFFIC DATA:									
Type of traffic data		ANNUAL AVERAGE DAILY TRAFFIC					DIRECTIONAL SPLIT		
CLASSIFIED-HOURLY		AADT		K-factor			Dir1 - Dir2		
(Class/Aadt/UNclass)		(veh/day)		(default: 0.11)			(default: 50 - 50)		
							NA - NA %		
Traffic Composition(%)	LV (%)	MHV (%)	LB (%)	LT (%)	MC (%)	Total (%)	LV = Light Vehicle	MHV = Medium Heavy Vehicle	LB = Large Bus
User values	31.67	34.16	0.134	3.597	30.43	100.0	LT = Large Truck	MC = MotorCycle	
(normal values)	(57.0)	(23.0)	(7.0)	(4.0)	(9.0)	(100.0)			

Traffic flow data for whole segment analysis:

Row	Dir	Light Vehicle	Med Heavy Veh	Large Bus	Large Truck	MotorCycle	Total flow Q	
1.1	Dir1	pce,1= 1.00	pce,1= 1.56	pce,1= 1.65	pce,1= 2.41	pce,1= 0.76		
1.2	Dir2	pce,2= 1.00	pce,2= 1.56	pce,2= 1.65	pce,2= 2.41	pce,2= 0.76		
		veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	Split veh/h pcu/h	
2	(1)	(2) (3)	(4) (5)	(6) (7)	(8) (9)	(10) (11)	(12) (13) (14)	
3	Dir1	484	484	624	976	2	3	
4	Dir2	458	458	392	613	2	3	
5	1+2	942	942	1016	1589	4	6	
6	Note.	If specific grade then				Directional split, SP= Q1/(Q1+Q2)=		54.9% 56.2%
7		dir 1 = uphill, dir 2= downhill				Pcu-factor, Fpcu =		1.172

SIDE FRICTION CLASS: If detailed data are available, use first table to determine weighted frequency of events and then go to second table. If not, use second table only.

1. Determination of frequency of events

Calculation of weighted frequency of events per hour and 200 m of the studied road segment.	Side friction type of events (20)	Symbol	Weighting factor (22)	Frequency of events (23)	Weighted frequency (24)
Frequencies are for both sides of the road.	Pedestrians	PED	0.6	NA / h,200m	NA
	Parking, stopping veh.	PSV	0.8	NA / h,200m	NA
	Entry+exit of vehicles	EEV	1.0	NA / h,200m	NA
	Slow-moving vehicles	SMV	0.4	NA / h	NA
Total:					NA

2. Determination of side friction class

Weighted frequency of events (30)	Typical conditions	Side friction class
< 50	Rural, agriculture or undeveloped with very few activities	VL= very low
50 - 149	Rural, some roadside buildings and some activities	L= low
150 - 249	Village, residential activities	M= medium
250 - 349	Village, some market activities	H= high
> 350	Almost urban, market and business activities	VH= very high
For current case indicate side friction class: L (L is default)		

KAJI -- INTERURBAN ROADS		Province:	JAWA TIMUR		Date:	JUNI 2014								
Form IR-3: Analysis		Link number:			Handled by:	ARIF								
		Segment code:			Checked by:	ARIF								
SPEED, CAPACITY		Administr. road class :	NATIONAL		Functional road class:	ARTERIAL								
Purpose: Operation		Road type :	4/2UD		Length (km) :	7.500								
		Time period :			Case number:									
FREE FLOW SPEEDS														
Option to enter other free flow speeds: No														
Di- rec- tion	Base free-flow speed FVo (km/h)		Carriage- way width adjust- ment, FVw Tab E2:1	FVo+FVw Light (2)+(3)	Adjustment factors		Actual free-flow speeds, km/h							
	for different vehicles Table B-1:1 or B-1:2				Side friction FFVsf Tab B3:1	Land use FFVrc Tab B4:1	FFVlv = (FVo+FVw)*FFVsf*FFVrc		Other vehicle types					
	LV	MHV	LB	LT			MC	(4)	(5)	(6)	(7)	MHV	LB	LT
	(2)	(2)	(3)	(3)	(4)	(5)	(6)	(7)	(4*5*6)					
1+2	74.0	63.0	78.0	60.0	60.0	-3.0	71.0	0.970	1.000	68.87	58.63	72.59	55.84	55.84
Comments:					User FFV, dir1: None!			dir2:						
CAPACITY														
Direc- tion	Base Capacity		Adjustment factors for capacity				Actual capacity, C							
	Co Table C-1:1 pcu/h (11)		Carriageway width FCw Table C-2:1 (12)		Directional split FCsp Table C-3:1 (13)		Side friction FCsf Table C-4:1 (14)		C= Co*FCw*FCsp*FCsf pcu/h (11)*(12)*(13)*(14) (15)					
1+2	6800		0.910		0.975		0.970		5854					
ACTUAL SPEED and TRAVEL TIME for light vehicles								Only 2/2UD roads						
Di- rec- tion	Traffic flow, Q Form IR-2 pcu/h (21)	Degree of saturation DS=Q/C (21)/(15) (22)	Actual speed, Vlv Fig D2:1/:2 km/h (23)	Road segment length, L km (24)	Travel time, TT (24/23) sec (25)	ACTUAL SPEEDS				Di- rec- tion	Degree of bunching DB Fig D3:1 (31)			
						for other vehicle types km/h								
1+2	3486	0.595	57.26	7.500	471.514	48.75	60.35	46.42	46.42					
Space for user remark:														
Program version 1.10F Date of run: 141015/9:23														

K A J I	Province	JAWA TIMUR	Date	JUNI 2014																									
INTERURBAN ROADS	Link number:		Handled by :	ARIF																									
	Segment code:		Checked by :	ARIF																									
Form IR-1: Input	Segment between	BULU and	TUBAN																										
	Specific grade:	No [NO indicates segment, YES spec grade(only 2/2UD)]																											
GENERAL DATA,	Administr. road class :	NATIONAL	Functional road class:	ARTERIAL																									
ROAD GEOMETRY	Road type :	4/2UD	Length (km) :	7.500																									
Purpose: Operation	Time period:		Case number:																										
HORIZONTAL ALIGNMENT																													
<pre> +---+ A * * * * * +---+ To: * * * * * To: <----- * * * * * BULU * * * * * * * * * * N Indicate * * * * * * * * * * +---+ B +--- north (N) * * * * * </pre>																													
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Program version 1.10F Date of run: 141015/9:24																													

KAJI -- INTERURBAN ROADS	Province: JAWA TIMUR	Date: JUNI 2014
Link number:	Handled by: ARIF	
Form IR-2: Input	Segment code:	Checked by: ARIF
TRAFFIC FLOW, SIDE FRICTION	Administr. road class : NATIONAL	Functional road class: ARTERIAL
	Road type : 4/2UD	Length (km) : 7.500
Purpose: Operation	Time period :	Case number:

TRAFFIC DATA:							
Type of traffic data	ANNUAL AVERAGE DAILY TRAFFIC		DIRECTIONAL SPLIT				
CLASSIFIED-HOURLY	ADDT	K-factor	Dir1 - Dir2				
(Class/Aadt/UNclass)	(veh/day)	(default: 0.11)	(default: 50 - 50)				
			NA - NA %				
Traffic Composition(%)	LV (%)	MHV (%)	LB (%)	LT (%)	MC (%)	Total (%)	LV = Light Vehicle
							MHV = Medium Heavy Vehicle
							LB = Large Bus
							LT = Large Truck
							MC = MotorCycle
User values	31.71	34.10	0.191	3.605	30.37	100.0	
(normal values)	(57.0)	(23.0)	(7.0)	(4.0)	(9.0)	(100.0)	

Traffic flow data for whole segment analysis:

Row	Dir	Light Vehicle	Med Heavy Veh	Large Bus	Large Truck	MotorCycle	Total flow Q							
1.1	rec	pce,1= 1.00	pce,1= 1.59	pce,1= 1.68	pce,1= 2.46	pce,1= 0.79								
1.2	tion	pce,2= 1.00	pce,2= 1.59	pce,2= 1.68	pce,2= 2.46	pce,2= 0.79								
		veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	veh/h pcu/h	Split veh/h pcu/h							
2	(1)	(2) (3)	(4) (5)	(6) (7)	(8) (9)	(10) (11)	(12) (13) (14)							
3	Dir1	511	511	656	1040	3	5	63	155	489	384	54.94	1722	2095
4	Dir2	483	483	413	655	3	5	50	123	463	363	45.05	1412	1629
5	1+2	994	994	1069	1695	6	10	113	278	952	747		3134	3724
6	Note.	If specific grade then				Directional split. SP= Q1/(Q1+Q2)=				54.9% 56.2%				
7		dir 1 = uphill, dir 2= downhill				Pcu-factor, Fpcu =				1.188				

SIDE FRICTION CLASS: If detailed data are available, use first table to determine weighted frequency of events and then go to second table. If not, use second table only.

1. Determination of frequency of events

Calculation of weighted frequency of events per hour and 200 m of the studied road segment.	Side friction type of events (20)	Symbol	Weighting factor (22)	Frequency of events (23)	Weighted frequency (24)
Frequencies are for both sides of the road.	Pedestrians	PED	0.6	NA / h,200m	NA
	Parking, stopping veh.	PSV	0.8	NA / h,200m	NA
	Entry+exit of vehicles	EEV	1.0	NA / h,200m	NA
	Slow-moving vehicles	SMV	0.4	NA / h	NA
Total:					NA

2. Determination of side friction class

Weighted frequency of events (30)	Typical conditions	Side friction class
< 50	Rural, agriculture or undeveloped with very few activities	VL= very low
50 - 149	Rural, some roadside buildings and some activities	L= low
150 - 249	Village, residential activities	M= medium
250 - 349	Village, some market activities	H= high
> 350	Almost urban, market and business activities	VH= very high
For current case indicate side friction class: L (L is default)		

KAJI -- INTERURBAN ROADS		Province:	JAWA TIMUR			Date:	JUNI 2014							
Form IR-3: Analysis		Link number:				Handled by:	ARIF							
		Segment code:				Checked by:	ARIF							
SPEED, CAPACITY		Administr. road class :	NATIONAL		Functional road class :	ARTERIAL								
Purpose: Operation		Road type :	4/2UD		Length (km) :	7.500								
		Time period :				Case number:								
FREE FLOW SPEEDS														
Option to enter other free flow speeds: No														
Di- rec- tion	Base free-flow speed FVo (km/h)					Carriage- way width adjust- ment, FVw Tab E2:1 (2)	FVo+FVw Light (2)+(3) (km/h)	Adjustment factors		Actual free-flow speeds, km/h				
	for different vehicles Table B-1:1 or B-1:2 LV MHV LB LT MC (2)							Side friction Tab B3:1 (4)	Land use FFVrc Tab B4:1 (6)	Light (4*5*6) (7)	Other vehicle types MHV LB LT MC			
1+2	74.0	63.0	78.0	60.0	60.0	-3.0	71.0	0.970	1.000	68.87	58.63	72.59	55.84	55.84
Comments:										User FFV, dir1: None! dir2:				
CAPACITY														
Direc- tion	Base Capacity		Adjustment factors for capacity					Actual capacity, C						
	Co Table C-1:1 pcu/h (11)		Carriageway width FCw Table C-2:1 (12)		Directional split FCsp Table C-3:1 (13)		Side friction FCsf Table C-4:1 (14)		C= Co*FCw*FCsp*FCsf pcu/h (11)*(12)*(13)*(14) (15)					
1+2	6800		0.910		0.975		0.970		5854					
ACTUAL SPEED and TRAVEL TIME for light vehicles										Only 2/2UD roads				
Di- rec- tion	Traffic flow, Q Form IR-2 pcu/h (21)	Degree of saturation DS=Q/C (21)/(15) (22)	Actual speed, Vlv Fig D2:1/:2 km/h (23)	Road segment length, L km (24)	Travel time, TT (24/23) sec (25)	ACTUAL SPEEDS				Di- rec- tion	Degree of bunching DB Fig D3:1 (31)			
						for other vehicle types km/h MHV LB LT MC								
1+2	3724	0.636	56.07	7.500	481.516	47.73	59.10	45.46	45.46					
Space for user remark:														
Program version 1.10F Date of run: 141015/9:24														

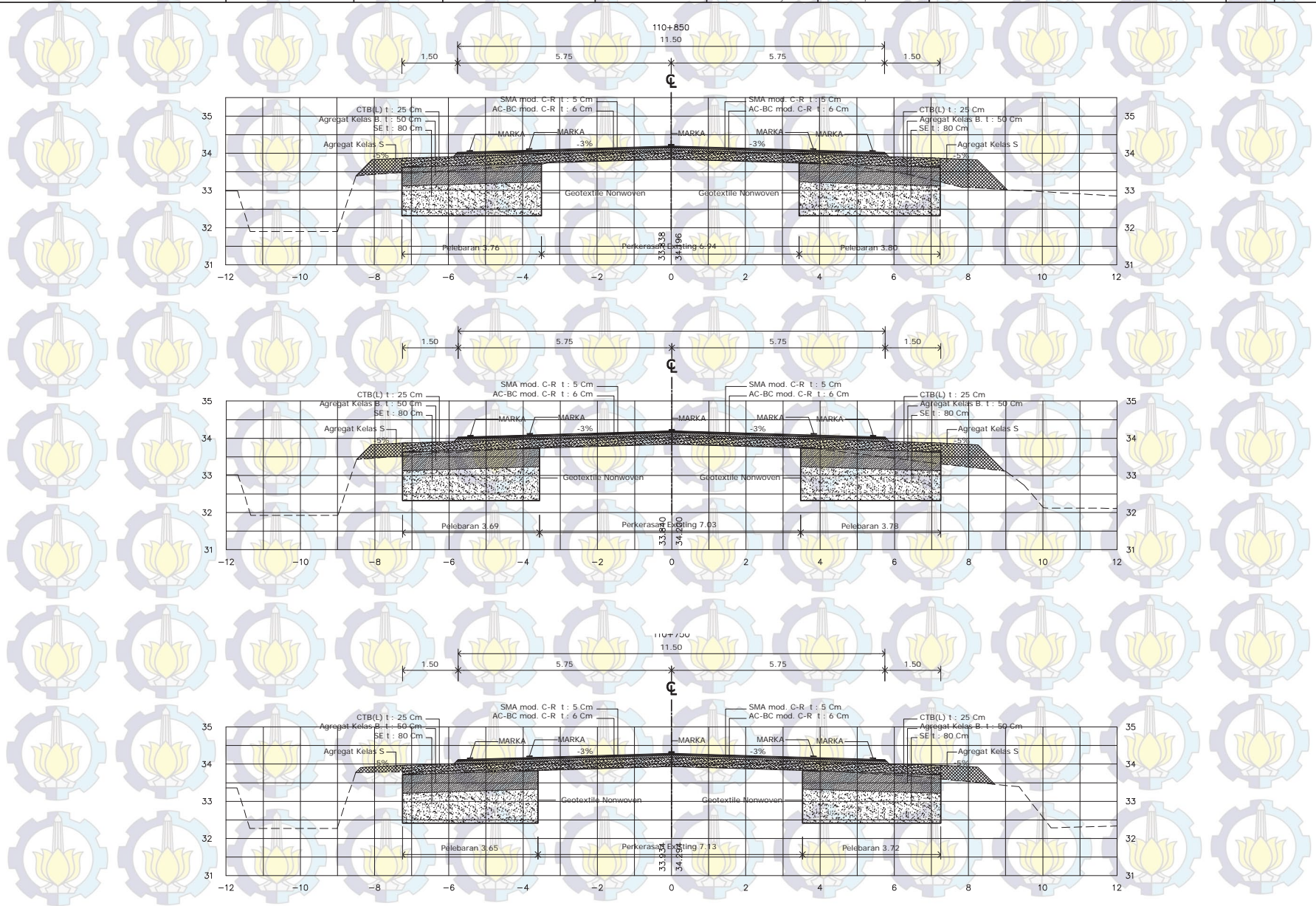


KEMENTERIAN PEKERJAAN UMUM
DIREKTORAT JENDERAL BINA MARGA
SNVT - PERENCANAAN DAN PENGAWASAN
JALAN DAN JEMBATAN JAWA TIMUR

Nama Proyek :	No. Paket	-
PEKERJAAN PERENCANAAN TEKNIK JALAN DAN JEMBATAN PROVINSI JATIM - 1	No. Link	-
Nama Paket / Link	BULU - BTS. KOTA TUBAN	
Provinsi	Jawa Timur	

Konsultan Perencana	PT. BUANA ARCHICON CORPORATE BUILDING & SOCIAL DEVELOPMENT	
Juru Gambarnya	Ir. DIDIK SULISTYA Atlit Jalan Raya	Ir. MUKHSIN BAZEID Kepala Tim

JUDUL GAMBAR :	No. Lembar	-
CROSS SECTION	Skala	H 1:100 - V 1:100
	Tanggal	-



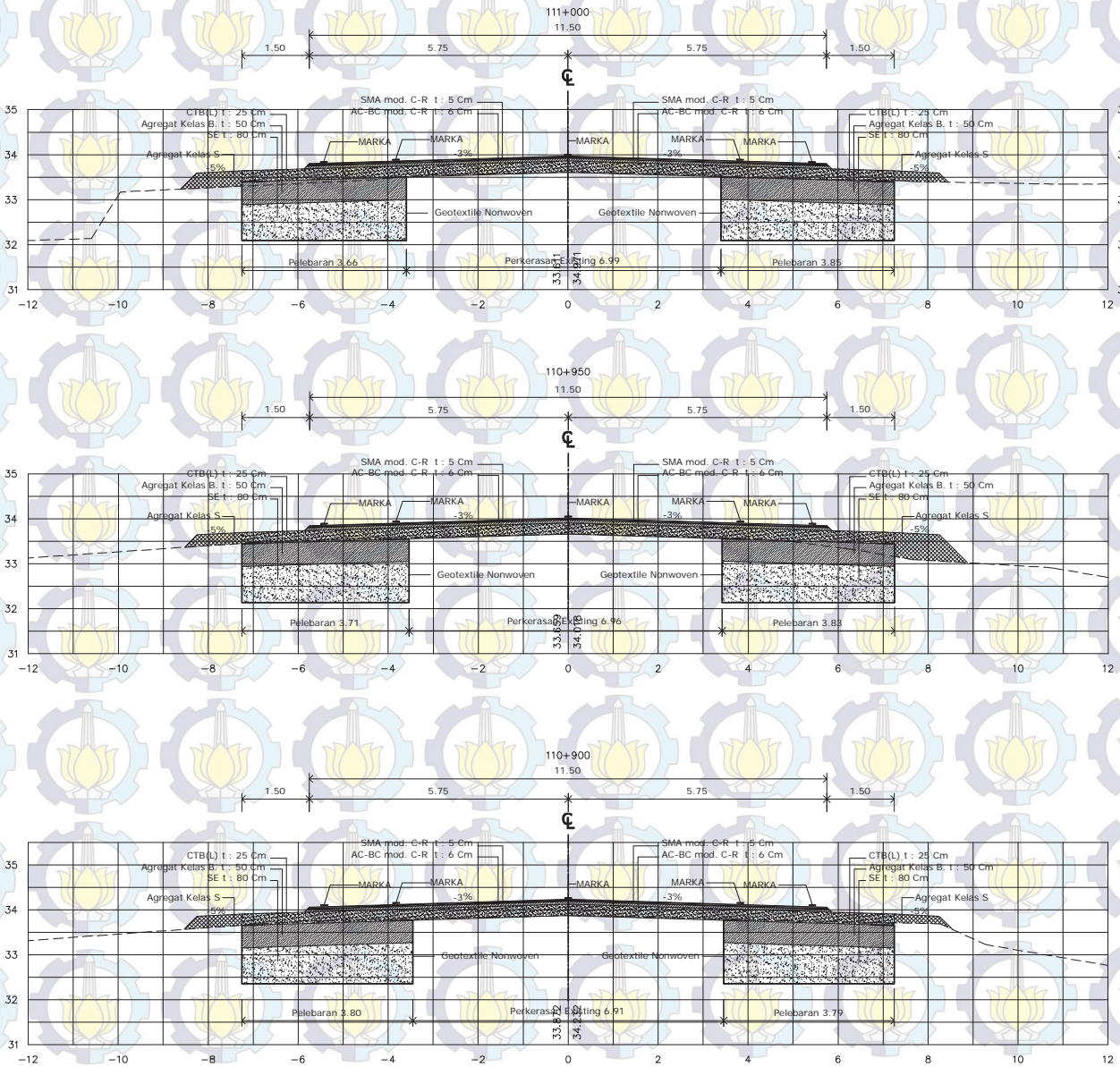


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	Provinsi	Jawa Timur

Konsultan Perencana	PT. BUANA ARCHICON CONSULTING, ENGINEERING, DESIGN DEVELOPMENT		
	<i>Joni Restiawan</i> JONI RESTIAWAN Juru Gambar	<i>Ir. Didik Sulisty</i> IR. DIDIK SULISTYA Atas Nama Rona	<i>Ir. Mukhsin Bazeid</i> IR. MUKHSIN BAZEID Kepala Tim

JUDUL GAMBAR :	No. Lembar	-
CROSS SECTION	Skala	H 1:100 - V 1:100
	Tanggal	-



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Kartika, A.A.G, ST, MSc. 2006. **Ekonomi Jalan Raya**. Diktat Kuliah, Program Studi S-1 Lintas Jalur Jurusan Teknik Sipil FTSP- ITS, Surabaya.

Tamin, O.Z. 2000. **Perencanaan dan Pemodelan Transportasi**. Bandung: ITB.

BIODATA PENULIS



Penulis dilahirkan di Malang, pada tanggal 22 Januari 1990, merupakan anak kedua dari empat bersaudara. Penulis telah menempuh pendidikan formal yaitu di SD Negeri Percobaan 1 Malang, SMP Negeri 4 Malang, dan SMA Negeri 2 Malang. Setelah lulus dari SMA Negeri 2 Malang, penulis melanjutkan pendidikan di Program DIII Teknik Sipil FTSP-ITS Surabaya dengan mengambil program studi Bangunan Transportasi pada tahun 2008 dan lulus pada tahun 2011. Kemudian pada tahun 2011 penulis melanjutkan ke Program Sarjana Lintas Jalur Teknik Sipil FTSP-ITS Surabaya. Di Jurusan Teknik Sipil ini, penulis mengambil judul Tugas Akhir di bidang Transportasi/ Perhubungan. Penulis bisa dihubungi lewat email fuzzywuzzy.150@gmail.com.